Alberta

## SOUTHERN REGION GRMP SITE INSPECTION FORM



SITE NUMBER AND NAME:		HIGHWAY & KM:		PREVIOUS INSPECTION D	INSPECTION DATE:	
S026 Elkwater Slides		41:03, 35.169		INSPECTION DATE: May 8. 201	May 8, 2019	
				May 3, 2018	····· <b>··</b>	
LEGAL DESCRIPTION	NAD 83 COORDINATES:			RISK ASSESMENT:		
SE-18-008-02 W4M and	UTM	Northing	Easting	Site A: PF: 9 CF: 6 TOTAL: 54 (Pile	e wall)	
16-07-008-02 W4M	12	5499046	553536	Site B: PF: 10 CF: 3 TOTAL: 30 (Slic	les)	
AVERAGE ANNUAL DAILY TRAFFIC (AADT): 710 (north), 500 (south), (Ref. No. 138060)				CONTRACTOR MAINTENANCE AREA (CMA): 23		

SUMMARY OF SITE INSTRUMENTATION:	INSPECTED BY:
Site A: Two piezometers and two slope inclinometers (SIs). The Measurand	Chris Gräpel (KCB)
ShapeAccelArray (SAA) is inoperable and the reading box was decommissioned in	Chris Morgan (KCB)
spring 2019 by KCB at the request of AT.	Alex Frotten (AT)
	Roger Skirrow (AT)
Site B: One SI	Nicolas Ropchan (AT)
	Doug Scheelar (AT)
LAST READING DATE: May 2019	

PRIMARY SITE ISSUE: Landsliding on fill slopes and back slopes on cut and fill side hill locations in a valley. Toe erosion from the creek and high groundwater table appear to be the triggering mechanisms.

APPROXIMATE DIMENSIONS: The extent of landsliding at this site is continuous over approximately 600 m from the south end to the north end of the site and between sites A and B, as previously identified in GRMP reports. The fill slopes vary from 5 to 10 m high, sloped at approximately 4H:1V to 5H:1V.

DATE OF ANY REMEDIAL ACTION: Shallow drainage installed near south end of site (in 1970s or 1980s, no records available). H-pile wall 60 m long constructed at Site B in 2012 (waypoints 601 and 602 mark the extents of the pile wall in Figure 1, the pile wall repair at this site was preceded by a temporary repair consisting of slope excavation and reconstruction and a soil nailing program). The slope at Site A was regraded in fall 2016 and new overlay placed in fall 2017. Numerous patches over the years.

ITEM	CONDITION EXISTS		DESCRIPTION AND LOCATION	NOTICABLE CHANGE FROM LAST INSPECTION	
	YES	NO		YES	NO
Pavement Distress	х		Overlay at Site A in fall 2017. Cracking in pavement has redeveloped at previous slide locations. The extent and width of cracking at Site B seems unchanged.		х
Slope Movement	x		Slope movement (translational) from Site A to the pile wall at the southern end of the site. At Site B, sliding is occurring below the pile wall, exposing the piles and causing loss of soil behind the pile wall.	х	
Erosion	osion X		Erosion on embankment slope at culvert outlet to south of side-hill site.		х
Seepage	x		Wet areas noted on back slopes (upslope of highway) and near toe of slope near stream (downslope of highway).		х
Culvert Distress		Х	Erosion at outlet of culvert		Х

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#### COMMENTS

Entire valley slope in the subject area is a landslide zone with instability features located upslope and downslope of the highway and a general trend of movement to the west, towards the creek. Numerous embankment fill slides are present to the west of road. Toe of landslide appears to be at creek level where erosion is occurring due to stream being partially blocked by slide movement. The general area of the site (Cypress Hills) was not glaciated in the last ice age.

Site A (embankment fill slides due to movement on the west side of the highway):

- Ongoing movement observed at the site since the 2018 inspection. Back slope slides are still active and snowplow strikes are visible on the road surface. Cracking has reflected through the recent overlay and settlement of the various slides is visible on the pavement surface.
- Fill settlement up to 0.5 m at east edge of pavement. Up to 1.0 m of asphalt exposed at shoulder of highway. Guard rail, HTCB of fill placement is required at locations where steep drop offs are located at the edge of the pavement, due to the narrow shoulder outside of the lanes and to protect road users.
- Water levels in the ditch downstream of the instability zones appears unchanged from 2018.
- Pavement cracking extending into centre of northbound lane at waypoint 603. Area of sliding is 40 m wide at edge of pavement of the southbound lane.
- The embankment slope below the highway between waypoints 604 and 607 is experiencing a series of semi-continuous slope failures:
  - Pavement cracking extending into southbound lane (to within 1 m of centreline) at waypoint 605.
    Area of sliding is 18 m wide at edge of pavement of the southbound lane.
  - Pavement cracking extending to 0.5 m past edge of pavement into southbound lane at waypoint 606. Area of sliding is 5 m wide at edge of pavement of the southbound lane.
  - Pavement cracking extending to 1.0 m past centreline into northbound lane at waypoint 607. Area of sliding is 25 to 30 m wide at edge of pavement of the southbound lane.

Site B (H-pile wall) - The pile wall at Site B has been exposed by soil sliding downslope of the wall. A small amount of ground cracking was observed between the highway and pile wall.

Pavement cracking upslope of the pile wall was noted at the southern end of the wall, where the wall ends. Cracking and slope movement is also present north of the northern extent of the pile wall, outflanking the pile wall.

Slope movements below pile wall have exposed the piles over a length of 30 m at the north limit of the wall. The piles were measured as leaning between 0.2° and 0.6° from vertical in 2018. The upper 2 to 3 m of pile wall is exposed and unsupported. Below the exposed section of H-piles is a 2 m high section where sloughed material from between and behind the pile wall has eroded out and covered the piles. Voids are up to 0.5 m deep behind the pile wall, and erosion is ongoing.

Well-developed toe roll near creek level beyond the edge of the trees below pile wall. Wet conditions present at toe of slope. Seepage and surface water runoff has resulted in exposed soils being eroded and deposited at toe of slope. The guardrail at the north edge of the pile wall has a noticeable dip.

Candidate repair options for this site are:

Site A – Carry out additional ground investigation, including boreholes and installation of additional geotechnical instrumentation, to assess the depth of movement in recently active areas for repair design options evaluation. Potential repair options include additional pile walls in slide areas, with drainage trenches installed at the toe of the landslide zone to lower the groundwater table and improve slope stability.

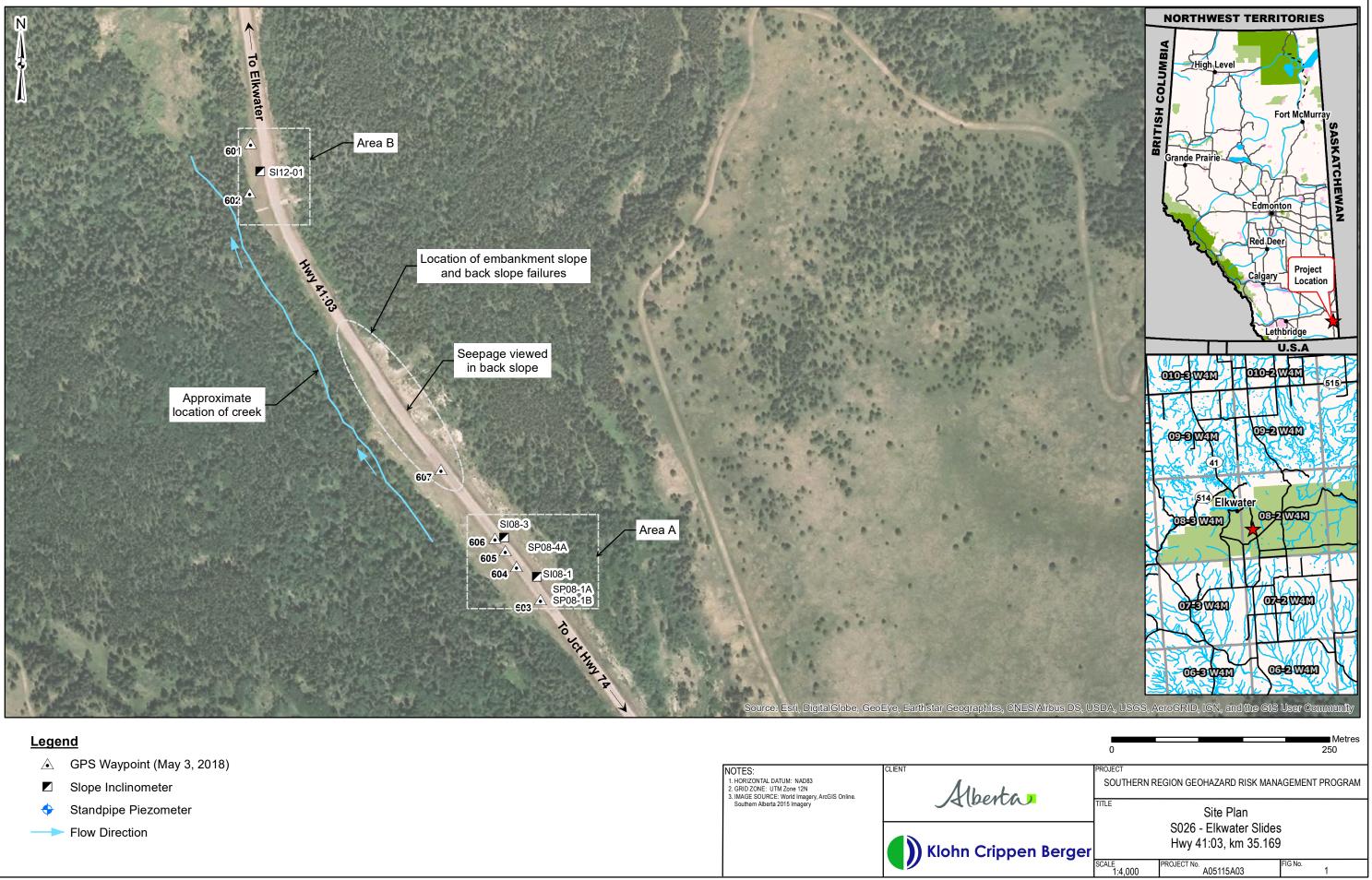
Site B - Candidate repairs include extending the existing H-pile wall to the north and the addition of pile wall facing (e.g., timber lagging) to minimize material loss between the piles. Drill boreholes and install two additional two slope inclinometers adjacent to the pile-wall.

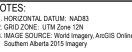
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Ditch drainage through the whole S026 area should be improved to minimize infiltration into the slide zones. The highway surface should continue be regraded in areas where it has deflected and cracked, to improve the smoothness of the highway through this site. Highway regrading should include milling the existing asphalt, and not placing more material which adds weight to the failure zones.





# Photo 1 Southbound lane cracking upslope of Area B pile wall. Pile wall located downslope to the west of the highway. Photo was taken facing north on May 8, 2019.



Photo 2 Dip in guard rail at Site B. Photo taken facing north on May 8, 2019.





Photo 3 Site B pile wall exposed length is approximately 30 m, with 2 to 3 m of pile wall exposed. Photo was taken facing northwest on May 8, 2019.



Photo 4 Exposed pile wall with 2 to 3 m of exposed H-pile and voids up to 0.5 m due to washout of material between piles. Photo taken facing north on May 8, 2019.





Photo 5 The Site B slide area extends past the north limit of the pile wall (red circle). Photo was taken facing downslope on May 8, 2019.



Photo 6 Back slope failure has blocked ditch, leading to standing water in ditch. Photo taken facing northeast on May 8, 2019.





Photo 7 Cracking has redeveloped through the pavement overlay at Site A. Photo was taken facing north on May 8, 2019.



Photo 8 Pavement cracking and settlement at Site A. Photo taken on May 8, 2019.





Photo 9 Narrow shoulder with fill slope dropping up to 0.5 m at east edge of pavement. Photo taken facing south on May 8, 2019.



Photo 10 Back scarp of an embankment slide area, on the west slope at Area A site. Photo taken facing west on May 8, 2019.



