# ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS GRMP PEACE REGION – (PEACE RIVER DISTRICT) FALL 2024



Site Number	Location	Name	Hwy	km
PH071 Hwy 986:01 C1 28.474		Daishowa West Hill	986:01	Km 28.5
Legal Description: 8-9-85-21 W5		UTM Co-ordinates		
		11U E 485023	N 624	45526

Current Monitoring:	19-Sep-2024	Previous Monitoring	20-May-2024	
Instruments Read By:	Mr. Niraj Regmi, G.	I.T and Mr. Nixson Mationg, of Thurber	r	

Instruments Read During This Site Visit							
Slope Inclinometers (SIs): SI16-3	Pneumatic Piezometers (PN):	Vibrating Wire Piezometers (VW): VW16-1A, VW16-1B, VW16-1C, VW16-2A, VW16-2B, VW16-2C, VW16-3A, VW16-3B, VW16-4A, VW16-5A	Standpipe Piezometers (SP):				
Load Cell (LC): VC1917, VC1918, VC1919, VC1920, VC1921, VC1922, VC1923, VC1924 VC1925	Strain Gauges: twenty-eight vibrating wire strain gauges	<b>SAAs:</b> SAA-P060, SAA-P097	Others:				

Readout Equipment Used							
Slope Inclinometers: RST Digital Inclinometer probe with a 2 ft wheelbase and a RST Pocket PC readout	Pneumatic Piezometers:	Vibrating Wire Piezometers: Campbell Scientific CR6 datalogger	Standpipe Piezometers:				
Load Cell: Campbell Scientific CR6 datalogger	<b>Strain Gauges:</b> Campbell Scientific CR6 datalogger	SAAs: Campbell Scientific CR6 datalogger	Others:				

Discussion					
Zones of New Movement:	None				
	Slope Indicators and SAAs				
	SI16-3 showed no discernible movement over 0.1 m to 3.8 m depth and 14.1 m to 16.0 m depth, respectively, since the spring of 2024 readings. SI16-3 showed a rate of movement of 0.2 mm/yr over 9.3 m to 11.1 m depth.				
Interpretation of Monitoring Results:	SAA-P060 showed a rate of movement of 1.6 mm/yr over 2.5 m to 29.5 m depth, and a rate of movement of 1.3 mm/yr over the full length of the pile and waler since the spring of 2024 readings. SAA-P060 has shown a cumulative pile head deflection of 18.5 mm to date in the downslope direction. SAA-P060 has shown an overall relatively steady rate of movement since the end of construction with a total incremental movement of about 6.5 mm in the downslope direction since completion of construction				
	SAA-P097 has shown no discernible movement since the spring of 2024 readings. SAA-P097 has shown a total pile head deflection of 17.5 mm in the downslope direction to date. SAA-P097 had shown an overall trend of steady				

Future Work:	The instruments should be read again in the spring of 2025. It is recommended to continue monitoring the deflections in the pile wall and the load cell readings to see if there are further increases in deflection or loads compared to the
	Since the end of construction, the load cells have generally shown an overall trend of increasing load, with the highest seasonal loads measured towards the end of each winter. The current loads measured in the upper two anchor rows at all three piles, as well as the lower row at P022, are above the design loads. However, the recent readings for P060 and P097 appear to show that the anchor loads in the upper two anchor rows in P060 and P097 are starting to level off.
	Load cells VC1924 (anchor P022A) and VC1923 (anchor P022B) lost function of a second vibrating wire channel during July 2024, and the readings reported were the most recent available reading.
	It should be noted that load cells VC1922, anchor P060B, and VC1918, anchor P097B, have each lost the function of one vibrating wire channel. The loads that are now reported for these three load cells are based on the average of the two functioning channels and projecting a reading for the now malfunctioning channel based on the last reading taken for that channel and the changes observed in the two functional channels since that time.
	The load cells generally showed small changes in measured load compared to the spring of 2024 readings, ranging from a decrease of 4.78 kN in VC1920 (anchor P060A to an increase of 3.90 kN in VC1922 (anchor P060B.
	Load Cells
	VW16-4A continued to be dry. VW16-5A was dry during the current readings. VW16-4A and VW16-5A tend to remain dry except for brief periods that correspond to either higher than normal rainfall or spring thaw.
	Of the piezometers located downslope of the wall, VW16-3A showed a decrease in groundwater level of 0.21 m, and VW16-3B showed no change in groundwater level since the spring of 2024 readings.
	VW16-2A continued to be dry (this piezometer has only shown a groundwater level above the tip during a period between August 9, 2022, and September 21, 2022).
	Vibrating wire piezometer VW16-1A showed a groundwater elevation of 488.07 m and was dry during the spring 2024 readings. VW16-1B, and VW16-1C, located upslope of the wall, showed an increase in groundwater level of 0.41 m and 0.04 m respectively since the spring of 2024 readings. VW16-2C and VW16-2B, also located upslope of the wall, showed increases in groundwater levels of 0.76 m and 0.50 m, respectively since the spring of 2024 readings. The groundwater level of 482.13 m, measured in VW16-2C on September 5, 2024 was the highest measured in the instrument since it was initialized. However, the overall trend since end of construction had been a gradual increase in the groundwater table upslope of the wall to what now appears to be a relatively steady level with cyclical seasonal fluctuations.
	Piezometers
	The strain gauge readings in Pile P066 are summarized in Table PH071-3. Overall, the strain gauges showed relatively small changes in microstrain value compared to the previous readings in the spring of 2023. The greatest change in microstrain was in strain gauge #17 at 2.6 m depth in the downslope face of the pile, which showed a decrease in positive (compressive) microstrain of 10.7 microstrain compared to the spring of 2024 readings.
	Strain Gauges
	downslope movement since the end of construction, with peaks of higher downhill movement rates during the winter months.

	warning threshold values. If the deflections and or load cell readings continue to increase, it may become necessary to add additional anchors to the pile wall. It is recommended to wait and see if load cells VC1924 (anchor P022A) and VC1923 (anchor P022B) regain functioning of one of their vibrating wire channels by the spring of 2025 readings. If only one vibrating wire channel is functioning going forwards, the readings should be reported based on only the working channel, and comparisons should be made based off this new estimated load. Alternatively, a projected load trend based on the historic data may need to be estimated for input into future wall performance assessments. The load cells at these anchors have not shown a clear trend of leveling off and projected loads are up to 50 kN above the design load level. With respect to these observations, a review of the current anchor load distribution could be undertaken to better assess the pile wall performance.
Instrumentation Repairs:	No instrument repairs are required at this time.
Additional Comments:	

	<ul> <li>Table PH071-1: Fall 2024 – HWY 986:01, Daishowa West Hill Slope Inclinometer Instrumentation Reading Summary</li> </ul>
	<ul> <li>Table PH071-2: Fall 2024 – HWY 986:01, Daishowa West Hill Shape Accelerometer Array Instrumentation Reading Summary</li> </ul>
	<ul> <li>Table PH071-3 Fall 2024 – HWY 986:01, Daishowa West Hill Vibrating Wire Strain Gauge Instrumentation Reading Summary</li> </ul>
	<ul> <li>Table PH071-4: Fall 2024 – HWY 986:01, Daishowa West Hill Vibrating Wire Piezometer Instrumentation Reading Summary</li> </ul>
	<ul> <li>Table PH071-5: Fall 2024 – HWY 986:01, Daishowa West Hill Load Cell Instrumentation Reading Summary</li> </ul>
	<ul> <li>Statement of Limitations and Conditions</li> </ul>
Attachments:	APPENDIX A - PH071 FALL 2024
	Field Inspector's report
	<ul> <li>Site Plan Showing Approximate Instrument Locations (Drawings No. 32121 PH071 1 and 32121-PH071-2)</li> </ul>
	SI Reading Plots
	SAA Reading Plots
	<ul> <li>Figure PH071-1 (Upslope Piezometer Elevations)</li> </ul>
	<ul> <li>Figure PH071-2 (Downslope Piezometer Elevations)</li> </ul>
	<ul> <li>Figure PH071-3 (Load Cell Data Pile P22)</li> </ul>
	Figure PH071-4 (Load Cell Data Pile P60)
	<ul> <li>Figure PH071-5 (Load Cell Data Pile P97)</li> </ul>

We trust this report meets your requirements at present. If you have any questions, please contact the undersigned at your convenience.

Yours very truly, Thurber Engineering Ltd. Roger Skirrow, M.Sc., P. Eng. Senior Geotechnical Engineer

Lucas Green, P.Eng. Geotechnical Engineer



### *Table PH071-1: Fall 2024 – HWY 986:01, Daishowa West Hill Slope Inclinometer Instrumentation Reading Summary* Date Monitored: September 19, 2024

INSTRUMENT #	DATE INITIALIZED	TOTAL CUMULATIVE RESULTANT MOVEMENT AT NOTED DEPTH SINCE INITIAL READING (mm)	MAXIMUM RATE OF MOVEMENT (mm/yr.)	CURRENT STATUS	DATE OF PREVIOUS READING	INCREMENTAL MOVEMENT SINCE PREVIOUS READING (mm)	RATE OF MOVEMENT (mm/yr.)	CHANGE IN RATE OF MOVEMENT SINCE PREVIOUS READING (mm/yr.)
	December 2, 2017 (Re-initialized)	32.5 over 0.1 m to 3.8 m depth in 101° direction	9.9 in October 2020			No discernible movement	N/A	-5.1
SI16-3		4.0 over 9.3 m to 11.1 m depth in 136° direction	1.8 in October 2020Operational1.2 in October 2020	Operational	May 20, 2024	<0.1	0.2	0.2
		2.8 over 14.1 m to 16.0 m depth in 101° direction			No discernible movement	N/A	-0.2	
SI16-4	September 6, 2016	9.7 over 10.4 m to 12.4 m depth	171.6 on September 15, 2016	Sheared at 11.4 m depth	September 30, 2016	N/A	N/A	N/A
SI16-5	August 30, 2016	64.5 over 8.0 m to 9.5 m depth	2404.8 on September 9, 2016	Sheared at 8.5 m depth	September 9, 2017	N/A	N/A	N/A

Drawings 32121-PH071-1 and 32121-PH071-2 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.



 Table PH071-2: Fall 2024 – HWY 986:01, Daishowa West Hill Shape Accelerometer Array Instrumentation Reading Summary

 Date Monitored: September 19, 2024

INSTRUMENT #	DATE INITIALIZED	TOTAL CUMULATIVE RESULTANT MOVEMENT AT NOTED DEPTH SINCE INITIAL READING (mm)	CURRENT STATUS	DATE OF PREVIOUS READING	INCREMENTAL MOVEMENT SINCE PREVIOUS READING (mm)	AVERAGE RATE OF MOVEMENT SINCE PREVIOUS READING <sup>(1)</sup> (mm/yr.)	CHANGE IN AVERAGE RATE OF MOVEMENT SINCE PREVIOUS READING (mm/yr.)
SAA-P022	March 17, 2016	5.1 over 0 m to 29.5 m depth	Not September 9, functioning <sup>(2)</sup> 2016	September 9,	N/A	N/A	N/A
SAA-FUZZ	March 17, 2010	6.7 over 2.5 m to 29.5 m depth		N/A	N/A	N/A	
	Marsh 47, 2040	18.5 over 2.5 m to 29.5 m depth		May 20, 2024	0.5	1.6	1.0
SAA-P060	March 17, 2016	25.8 over 0.0 m to 29.5 m depth	Operational	May 20, 2024	0.4	1.3	0.1
SAA-P097	March 17, 2016	17.5 over 2.5 m to 29.5 m depth	Operational	May 20, 2024	No discernible movement	N/A	-1.8

Drawings: 32121-PH071-1 and 32121-PH071-2 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.

Notes:

1. SAA readings are recorded once per day by the on-site datalogger. Average movement rates are compared to the previous reading event.

2. SAA-P022 stopped functioning on September 13, 2016



 Table PH071-3: Fall 2024 – HWY 986:01, Daishowa West Hill Vibrating Wire Strain Gauge Instrumentation Reading Summary

 Date Monitored: September 19, 2024

DEPTH FROM TOP OF PILE P066 (m)	GAUGE #	TOTAL MICROSTRAIN (MAY 20, 2024) (με)	CHANGE IN MICROSTRAIN SINCE PREVIOUS READING (OCT. 10, 2023) (με)	MEASURED TEMPERATURE (°¢)	GAUGE #	TOTAL MICROSTRAIN (SEP 20, 2024) (με)	CHANGE IN MICROSTRAIN SINCE PREVIOUS READING (MAY 20, 2024) (με)	MEASURED TEMPERATURE (°¢)
	UPSLOPE PILE FACE					DOWN	SLOPE PILE FACE	
0.575	11	-107.5	-5.3	20.0	16	N/A	N/A	N/A
2.575	12	-47.9	-0.4	18.5	17	146.9	-10.7	20.3
4.575	13	-52.2	5.3	13.3	18	450.1	-4.2	13.3
6.575	14	-40.3	1.9	9.5	19	437.5	2.2	9.5
8.575	15	-43.0	1.3	-95.1	20	N/A	N/A	N/A
11.075	1	-9.9	-9.3	7.3	6	N/A	N/A	N/A
13.075	2	29.0	0.0	7.2	7	-67.5	2.3	-144.6
15.075	3	32.1	-0.5	7.2	8	166.8	-0.1	7.3
17.075	4	37.7	0.1	5.1	9	165.9	0.1	7.3
19.075	5	-3.8	-0.3	7.3	10	N/A	N/A	N/A
21.075	21	24.3	0.1	-136.6	22	192.3	-0.4	7.2
23.075	23	-25.6	0.1	7.1	24	117.2	-0.4	7.2
25.075	25	13.4	0.1	7.1	26	N/A	N/A	7.1
27.075	27	N/A	N/A	N/A	28	583.8	-0.3	7.0

Drawings 32121-PH071-1 and 32121-PH071-2 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.

Note: Strain gauges number 6, 10, 16, 20, 26 and 27 are not functioning



 Table PH071-4: Fall 2024 – HWY 986:01, Daishowa West Hill Vibrating Wire Piezometer Instrumentation Reading Summary

 Date Monitored: September 19, 2024

INSTRUMENT	DATE	TIP ELEV. (m)	GROUND ELEV. (m)	CURRENT STATUS	HIGHEST RECORDED GROUNDWATER ELEVATION (m)	CURRENT GROUNDWATER ELEVATION (m)	PREVIOUS GROUNDWATER ELEVATION (m) (MAY 20, 2024)	CHANGE IN WATER LEVEL SINCE PREVIOUS READING (m)
VW16-1A	July 27, 2016	487.04	491.04	Active	488.65 on November 10, 2023	488.07	Below Tip Elevation (Dry)	N/A
VW16-1B	July 27, 2016	482.04	491.04	Active	485.47 on November 10, 2023	485.20	484.79	0.41
VW16-1C	July 27, 2016	473.04	491.04	Active	482.61 on November 10, 2023	481.44	481.40	0.04
VW16-2A	April 26, 2017	484.73	487.73	Active	484.82 on September 9, 2022	Below Tip Elevation (Dry)	Below Tip Elevation (Dry)	N/A
VW16-2B	April 26, 2017	478.73	487.73	Active	484.20 on September 9, 2022	483.85	483.09	0.76
VW16-2C	April 26, 2017	469.73	487.73	Active	482.13 on September 5, 2024	482.09	481.59	0.50
VW16-3A	September 7, 2016	477.49	482.99	Active	481.87 on July 4, 2024	481.11	481.32	-0.21
VW16-3B	September 7, 2016	464.99	482.99	Active	470.25 on September 7, 2016	465.88	465.88	0
VW16-4A	August 30, 2016	470.99	481.43	Active	475.30 on June 18, 2020	Below Tip Elevation (Dry)	Below Tip Elevation (Dry)	N/A
VW16-4B	August 30, 2016	457.24	481.43	Not functioning	Below Tip Elevation (Dry)	N/A	N/A	N/A
VW16-5A	August 30, 2016	468.02	471.02	Active	470.09 on March 22, 2019	Below Tip Elevation (Dry)	Below Tip Elevation (Dry)	N/A
VW16-5B	August 30, 2016	452.52	471.02	Not functioning	452.60 on September 12, 2016	N/A	N/A	N/A

Drawings 32121-PH071-1 and 32121-PH071-2 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.



# Table PH071-5: Fall 2024 – HWY 986:01, Daishowa West Hill Load Cell Instrumentation Reading Summary Date Monitored: September 19, 2024

19, 2024					
ANCHOR NUMBER	DESIGN LOAD / LOCK-OFF LOAD (kN)	MAXIMUM RECORDED LOAD (kN)	RECORDED LOAD <sup>(1)</sup> (kN)	PREVIOUS RECORDED LOAD <sup>(1)</sup> (MAY 20, 2024) (kN)	CHANGE IN LOAD SINCE PREVIOUS READING (kN)
P022A	230/184	March 14, 2023	261.34 (July 25, 2024) <sup>(2)</sup>	263.35 <sup>(2)</sup>	-2.01
P022B	230/184	286.32 <sup>(3)</sup> on March 23, 2023	278.12(July 16, 2024) <sup>(3)</sup>	274.31 <sup>(3)</sup>	3.81
P022C	230/184	239.89 on February 1, 2024	237.97 (September 19, 2024)	236.06	1.91
P060A	230/184	288.90 on February 3, 2022	265.93 (September 19, 2024)	270.71	-4.78
P060B	230/184	301.83 on January 28, 2022	288.01 (September 19, 2024) <sup>(5)</sup>	284.11 <sup>(5)</sup>	3.90
P060C	230/184	236.85 on February 1, 2024	235.62 (September 19, 2024)	234.09	1.53
P097A	230/184	303.66 on March 14, 2022	242.62 (September 19, 2024)	246.68	-4.06
P097B	230/184	293.16 on March 9, 2022	254.97 (September 19, 2024) <sup>(4)</sup>	253.20 <sup>(4)</sup>	1.77
P097C	230/184	227.06 on March 17, 2022	221.15 (September 19, 2024)	220.22	0.93
	ANCHOR NUMBER P022A P022B P022C P060A P060B P060C P097A P097B	ANCHOR NUMBER         DESIGN LOAD /LOCK-OFF LOAD (kN)           P022A         230/184           P022B         230/184           P022C         230/184           P060A         230/184           P060B         230/184           P060C         230/184           P097A         230/184           P097B         230/184	ANCHOR NUMBER         DESIGN LOAD /LOCK-OFF LOAD (kN)         MAXIMUM RECORDED LOAD (kN)           P022A         230/184         281.69 <sup>(2)</sup> on March 14, 2023           P022B         230/184         286.32 <sup>(3)</sup> on March 23, 2023           P022C         230/184         239.89 on February 1, 2024           P060A         230/184         288.90 on February 3, 2022           P060B         230/184         301.83 on January 28, 2022           P060C         230/184         303.66 on March 14, 2022           P097A         230/184         293.16 on March 9, 2022           P097B         230/184         293.16 on March 9, 2022	ANCHOR NUMBER         DESIGN LOAD /LOCK-OFF LOAD (kN)         MAXIMUM RECORDED LOAD (kN)         RECORDED LOAD <sup>(1)</sup> (kN)         RECORDED LOAD <sup>(1)</sup> (kN)           P022A         230/184         281.69 <sup>(2)</sup> on March 14, 2023         261.34 (July 25, 2024) <sup>(2)</sup> P022B         230/184         286.32 <sup>(3)</sup> on March 23, 2023         278.12(July 16, 2024) <sup>(3)</sup> P022C         230/184         239.89 on February 1, 2024         237.97 (September 19, 2024)           P060A         230/184         288.90 on February 3, 2022         265.93 (September 19, 2024)           P060B         230/184         301.83 on January 28, 2022         288.01 (September 19, 2024) <sup>(5)</sup> P060C         230/184         303.66 on March 14, 2022         235.62 (September 19, 2024)           P097A         230/184         303.66 on March 14, 2022         242.62 (September 19, 2024)           P097B         230/184         293.16 on March 9, 2022         254.97 (September 19, 2024) <sup>(4)</sup> P097C         230/184         227.06 on March 9, 2022         221.15 (September 19, 2024) <sup>(4)</sup>	ANCHOR NUMBER         DESIGN LOAD /LOCK-OFF LOAD (kN)         MAXIMUM RECORDED LOAD (kN)         RECORDED LOAD <sup>(1)</sup> (kN)         PREVIOUS RECORDED LOAD <sup>(1)</sup> (kN)           P022A         230/184         281.69 <sup>(2)</sup> on March 14, 2023         261.34 (July 25, 2024) <sup>(2)</sup> 263.35 <sup>(2)</sup> P022B         230/184         286.32 <sup>(3)</sup> on March 23, 2023         278.12(July 16, 2024) <sup>(3)</sup> 274.31 <sup>(3)</sup> P022C         230/184         239.89 on February 1, 2024         237.97 (September 19, 2024)         236.06           P060A         230/184         288.90 on February 3, 2022         265.93 (September 19, 2024)         270.71           P060B         230/184         236.85 on February 1, 2024         286.01 (September 19, 2024) <sup>(5)</sup> 284.11 <sup>(5)</sup> P060C         230/184         236.85 on February 1, 2024         235.62 (September 19, 2024) <sup>(6)</sup> 234.09           P097A         230/184         303.66 on March 14, 2022         242.62 (September 19, 2024) <sup>(4)</sup> 246.68           P097B         230/184         293.16 on March 9, 2022         254.97 (September 19, 2024) <sup>(4)</sup> 253.20 <sup>(4)</sup> P097C         230/184         293.16 on March 9, 2022         221.15 (September 19, 2024) <sup>(4)</sup> 220.22

Drawings 32121-PH071-1 and 32121-PH071-2 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.

Notes:

1) Load cell data is recorded daily with the datalogger on site. Dataloggers are downloaded twice annually during instrumentation readings. See figures PH071-3, PH071-4, and PH071-5 in Appendix A for combined historical instrument readings.



- 2) Load is based on reading from July 25, 2024. As of September 16, 2021, only two wire channels are functional on load cell VC1924. The reported loads after this date are based on the average of the two functional channels and a projected reading for the now malfunctional channel. After July 25, only one wire channel was functional, so July 25, 2024 reading was used for comparison.
- 3) Load is based on reading from July 16, 2024. As of October 4, 2021, only two wire channels are functional on load cell VC1923. The reported loads after this date are based on the average of the two functional channels and a projected reading for the now malfunctional channel. After July 25, only one wire channel was functional, so July 16, 2024 reading was used for comparison.
- 4) As of May 15, 2022, only two wire channels are functional on load cell VC1918. The reported loads after this date are based on the average of the two functional channels and a projected reading for the now malfunctional channel.
- 5) As of December 5, 2023, only two wire channels are functional on load cell VC1922. The reported loads after this date are based on the average of the two functional channels and a projected reading for the now malfunctional channel.



### STATEMENT OF LIMITATIONS AND CONDITIONS

#### 1. STANDARD OF CARE

This Report has been prepared in accordance with generally accepted engineering or environmental consulting practices in the applicable jurisdiction. No other warranty, expressed or implied, is intended or made.

### 2. COMPLETE REPORT

All documents, records, data and files, whether electronic or otherwise, generated as part of this assignment are a part of the Report, which is of a summary nature and is not intended to stand alone without reference to the instructions given to Thurber by the Client, communications between Thurber and the Client, and any other reports, proposals or documents prepared by Thurber for the Client relative to the specific site described herein, all of which together constitute the Report.

IN ORDER TO PROPERLY UNDERSTAND THE SUGGESTIONS, RECOMMENDATIONS AND OPINIONS EXPRESSED HEREIN, REFERENCE MUST BE MADE TO THE WHOLE OF THE REPORT. THURBER IS NOT RESPONSIBLE FOR USE BY ANY PARTY OF PORTIONS OF THE REPORT WITHOUT REFERENCE TO THE WHOLE REPORT.

### 3. BASIS OF REPORT

The Report has been prepared for the specific site, development, design objectives and purposes that were described to Thurber by the Client. The applicability and reliability of any of the findings, recommendations, suggestions, or opinions expressed in the Report, subject to the limitations provided herein, are only valid to the extent that the Report expressly addresses proposed development, design objectives and purposes, and then only to the extent that there has been no material alteration to or variation from any of the said descriptions provided to Thurber, unless Thurber is specifically requested by the Client to review and revise the Report in light of such alteration or variation.

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### 5. INTERPRETATION OF THE REPORT

- a) Nature and Exactness of Soil and Contaminant Description: Classification and identification of soils, rocks, geological units, contaminant materials and quantities have been based on investigations performed in accordance with the standards set out in Paragraph 1. Classification and identification of these factors are judgmental in nature. Comprehensive sampling and testing programs implemented with the appropriate equipment by experienced personnel may fail to locate some conditions. All investigations utilizing the standards of Paragraph 1 will involve an inherent risk that some conditions will not be detected and all documents or records summarizing such investigations will be based on assumptions of what exists between the actual points sampled. Actual conditions may vary significantly between the points investigated and the Client and all other persons making use of such documents or records with our express written consent should be aware of this risk and the Report is delivered subject to the express condition that such risk is accepted by the Client and such other persons. Some conditions are subject to change over time and those making use of the Report should be aware of this possibility and understand that the Report only presents the conditions at the sampled points at the time of sampling. If special concerns exist, or the Client has special considerations or requirements, the Client should disclose them so that additional or special investigations may be undertaken which would not otherwise be within the scope of investigations made for the purposes of the Report.
- b) Reliance on Provided Information: The evaluation and conclusions contained in the Report have been prepared on the basis of conditions in evidence at the time of site inspections and on the basis of information provided to Thurber. Thurber has relied in good faith upon representations, information and instructions provided by the Client and others concerning the site. Accordingly, Thurber does not accept responsibility for any deficiency, misstatement or inaccuracy contained in the Report as a result of misstatements, omissions, misrepresentations, or fraudulent acts of the Client or other persons providing information relied on by Thurber. Thurber is entitled to rely on such representations, information and instructions and is not required to carry out investigations to determine the truth or accuracy of such representations, information and instructions.
- c) Design Services: The Report may form part of design and construction documents for information purposes even though it may have been issued prior to final design being completed. Thurber should be retained to review final design, project plans and related documents prior to construction to confirm that they are consistent with the intent of the Report. Any differences that may exist between the Report's recommendations and the final design detailed in the contract documents should be reported to Thurber immediately so that Thurber can address potential conflicts.
- d) Construction Services: During construction Thurber should be retained to provide field reviews. Field reviews consist of performing sufficient and timely observations of encountered conditions in order to confirm and document that the site conditions do not materially differ from those interpreted conditions considered in the preparation of the report. Adequate field reviews are necessary for Thurber to provide letters of assurance, in accordance with the requirements of many regulatory authorities.

### 6. RELEASE OF POLLUTANTS OR HAZARDOUS SUBSTANCES

Geotechnical engineering and environmental consulting projects often have the potential to encounter pollutants or hazardous substances and the potential to cause the escape, release or dispersal of those substances. Thurber shall have no liability to the Client under any circumstances, for the escape, release or dispersal of pollutants or hazardous substances, unless such pollutants or hazardous substances have been specifically and accurately identified to Thurber by the Client prior to the commencement of Thurber's professional services.

### 7. INDEPENDENT JUDGEMENTS OF CLIENT

The information, interpretations and conclusions in the Report are based on Thurber's interpretation of conditions revealed through limited investigation conducted within a defined scope of services. Thurber does not accept responsibility for independent conclusions, interpretations, interpretations and/or decisions of the Client, or others who may come into possession of the Report, or any part thereof, which may be based on information contained in the Report. This restriction of liability includes but is not limited to decisions made to develop, purchase or sell land.



# ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS GRMP (CON0022164) PEACE REGION (PEACE RIVER DISTRICT) INSTRUMENTATION MONITORING RESULTS

# FALL 2024

### APPENDIX A DATA PRESENTATION

SITE PH071: HWY 986:01, DAISHOWA WEST HILL

#### ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS PEACE REGION (PEACE RIVER DISTRICT) INSTRUMENTATION MONITORING FIELD SUMMARY (PH071) SPRING 2024

Location: Daishowa West Hill (Hwy 986:01 C1 28.474)	Readout:	
File Number: 32121	Casing: 3.34	
Probe: RST Set 8R	<b>Temp:</b> 20	
Cable: RST Set 8R	Read by: NRM/NKR	

#### SLOPE INCLINOMETER (SI) READINGS

SI#	GPS I	Location	Date	Stickup	Depth from top	Magn. North		Current Bot	tom		Probe/		Remarks
	(UT	M 11)		(m)	of Casing (ft)	A+ Groove		Depth Read	ings		Reel		
	Easting (m)	Northing (m)					A+	A-	B+	B-	#	Size (")	
SI16-3	485023	6245526	19-Sep-24	0.8	130 to 2	95	184	-174	27	-27	8R/8R	3.34	

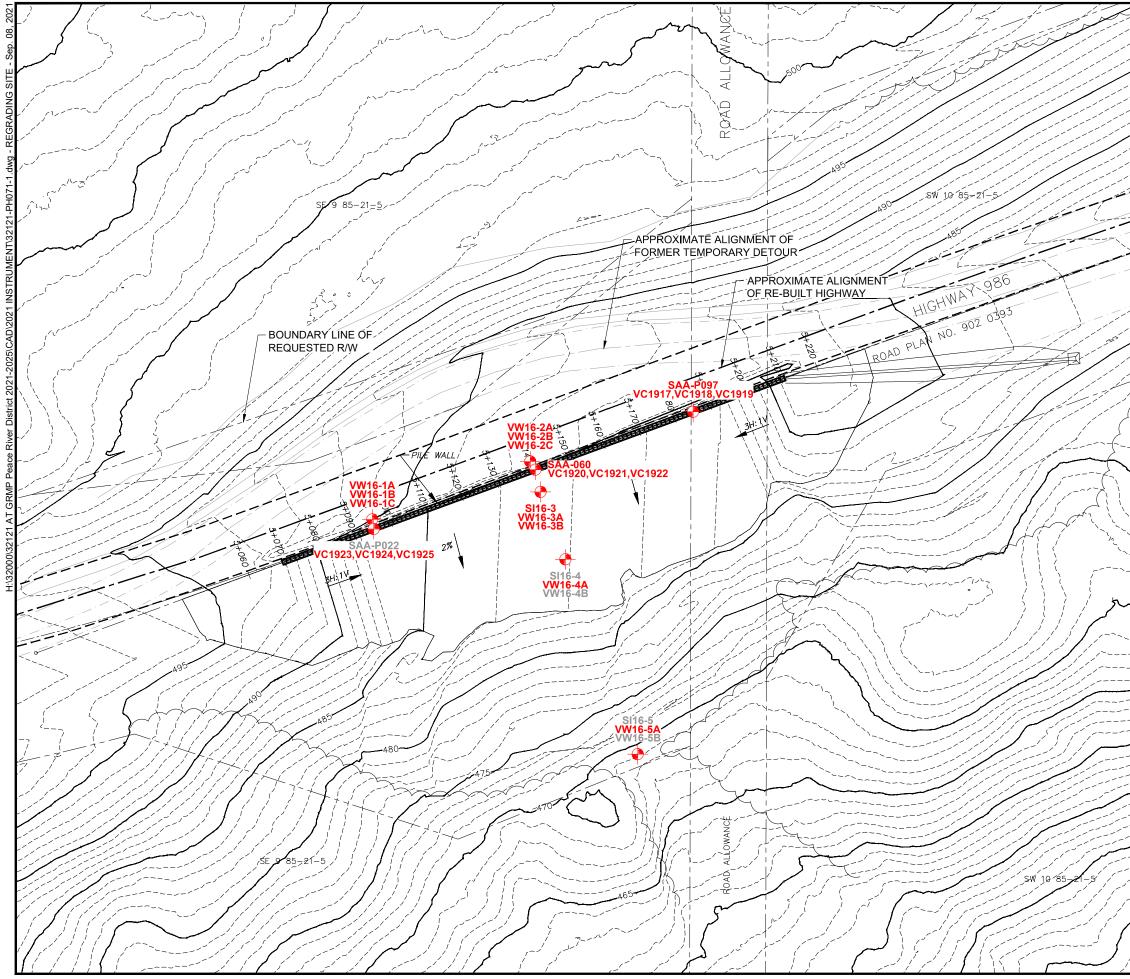
#### VIBRATING WIRE PIEZOMETER (VW) READINGS

VW #	Serial #	GPS Location (UTM 11)		Datalogger	Date	Comment	
		Easting (m)	Northing (m)	Serial #			
VW16-3A	VW36116	485022.21	6245527.86	RST 4885		Downloaded	
VW16-3B	VW36119	485022.21	6245527.86	K31 4005		Downloaded	
VW16-4A	VW36118	485028.75	6245510.02	RST 4915	19-Sep-24	Downloaded	
VW16-4B	VW36121	485028.75	6245510.02	K31 4915	19-3ep-24	Not Working	
VW16-5A	VW36117	485047.91	6245458.47	RST 4916		Downloaded	
VW16-5B	VW36120	485047.91	6245458.47	KS1 4910		Not Working	

#### INSPECTOR REPORT

CR6 datalogger is connected to a modem and does not need to be downloaded. CR6 datalogger is uesd to read Shape Accelerometer Array SAA-P060, SAA-P097, Vibrating Wire Piezometers VW16-1A to VW16-2C, and Load Cells VC1917 to VC1925.

Note: Download data from RST loggers. Do not take manual readings from VW16-4A/B







### LEGEND

+	APPROXIMATE INSTRUMENT LOCATION
$\sim$	TREE LINE
	GROUND CONTOURS
SI	SLOPE INCLINOMETER
VW	VIBRATING WIRE PIEZOMETER
VC	VIBRATING WIRE LOAD CELL
SAA	SHAPE ACCELEROMETER ARRAY

### NOTES:

- 1. TOPOGRAPHIC SURVEY AND COORDINATE GEOMETRY INFORMATION AS OF DECEMBER 2015 AS SUPPLIED BY WSP.
- 2. CONTOUR LINES ARE AT 1 m INTERVALS, MAJOR CONTOURS ARE LABELED AT FIVE METRE INTERVAL UNLESS NOTED OTHERWISE.
- 3. ALL DIMENSIONS ARE IN METRES UNLESS NOTED OTHERWISE.

0	10	20	30	40	50	<u>60</u> m
		SCA	LE 1:10	00		

BASE PLAN PROVIDED BY WSP

Alberta

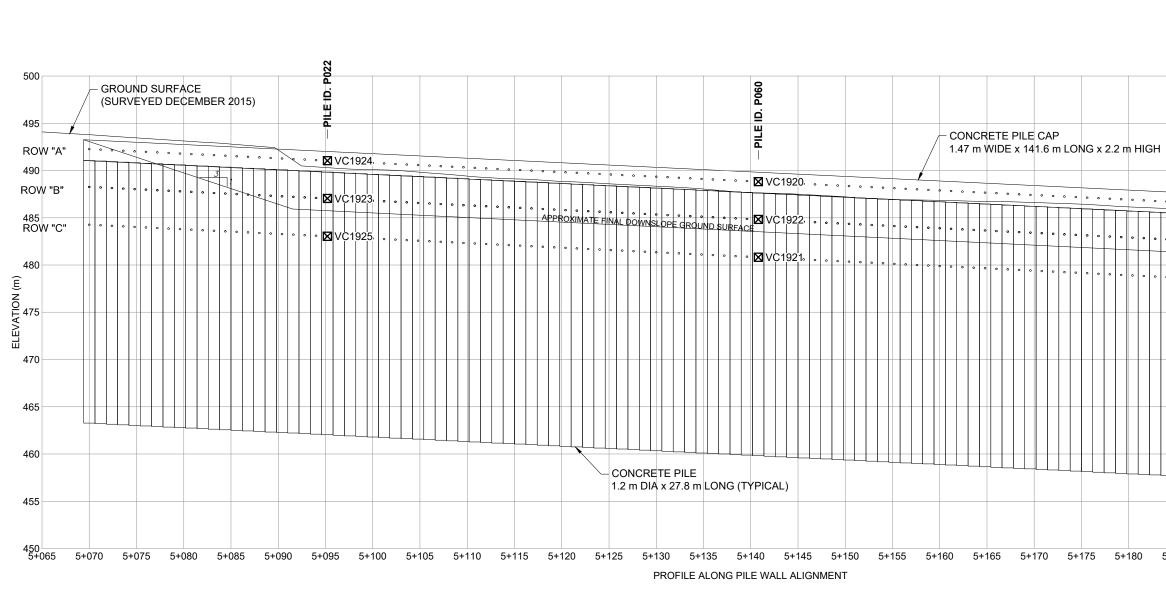
# PEACE REGION (PEACE RIVER DISTRICT)

# PH071: PEACE RIVER DIASHOWA WEST HILL INSTRUMENTATION READINGS

DWG No. 32121-PH071-1

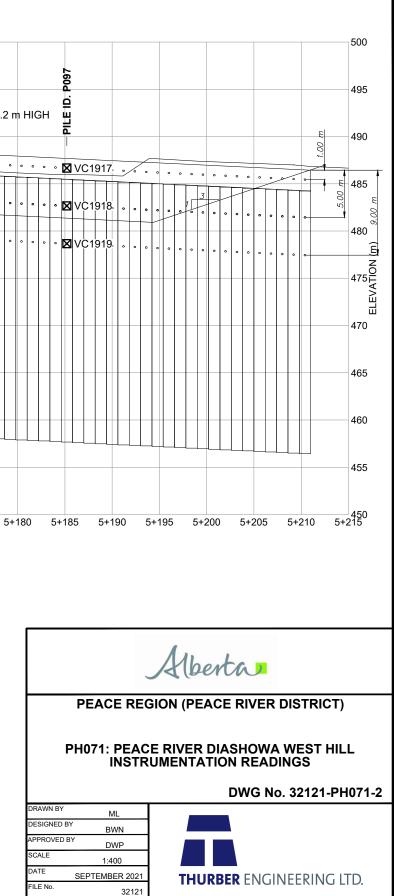
,-	DRAWN BY	ML
	DESIGNED BY	BWN
,	APPROVED BY	DWP
·	SCALE	1:1000
	DATE	SEPTEMBER 2021
Ć	FILE No.	32121

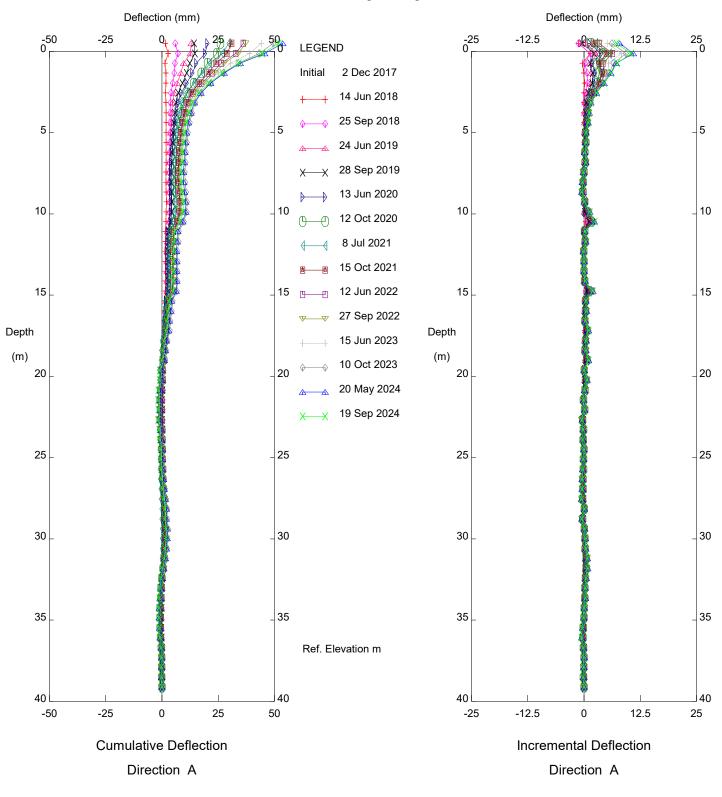


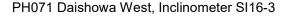


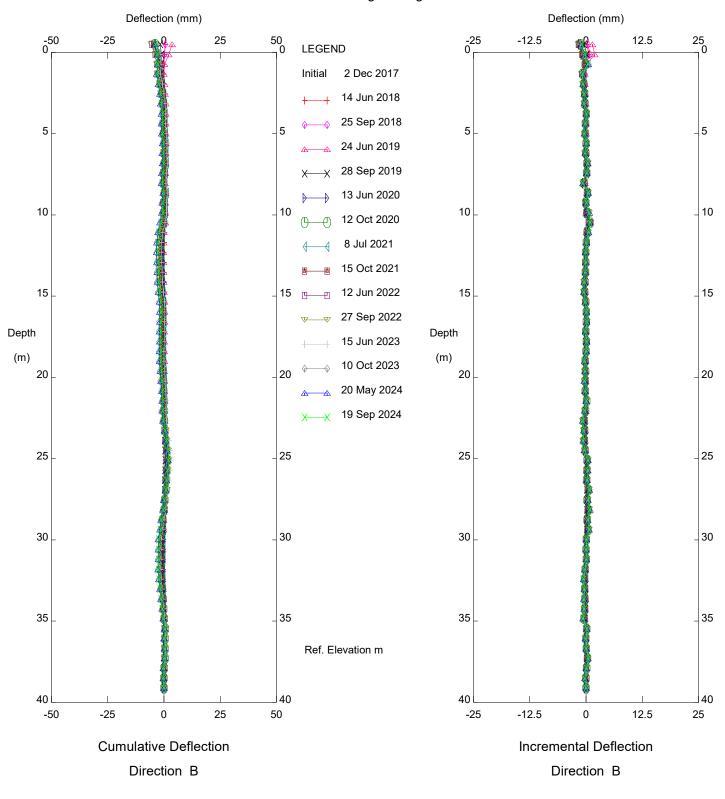
LEGEND

LOAD CELL LOCATION

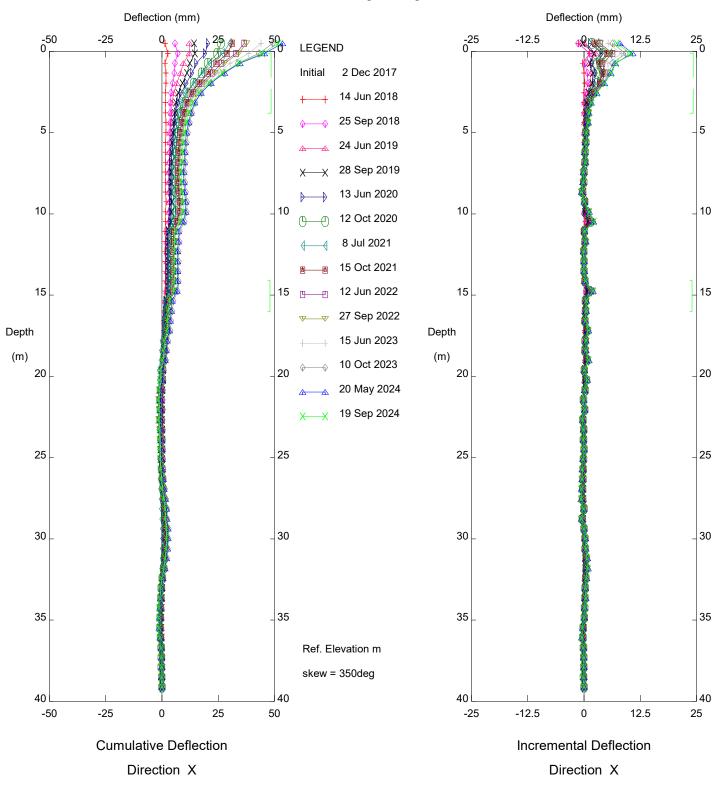




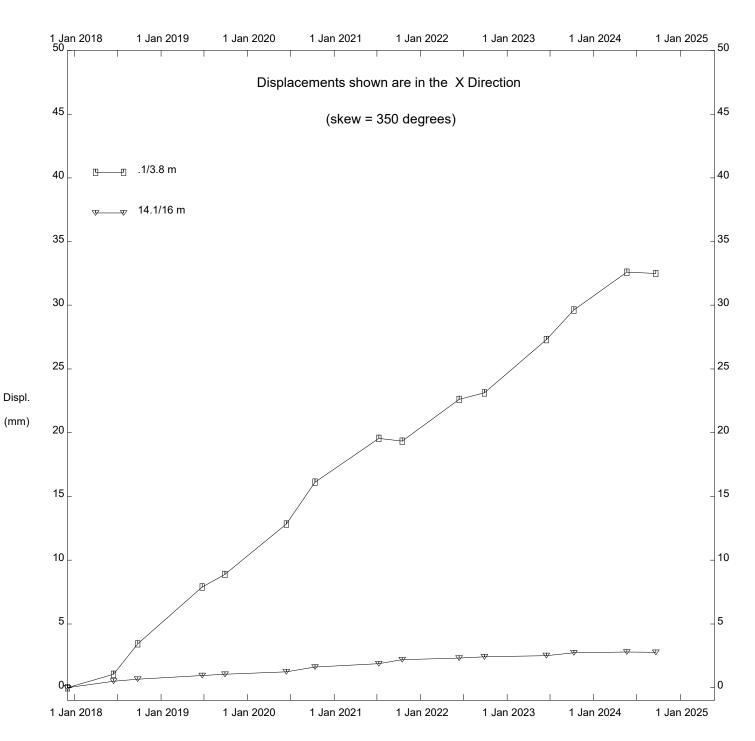




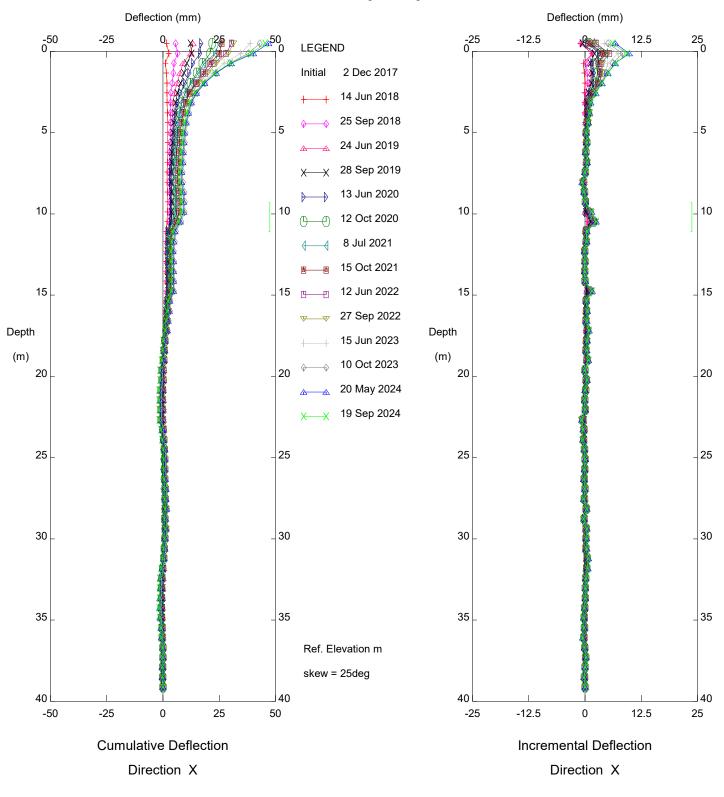


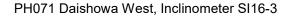


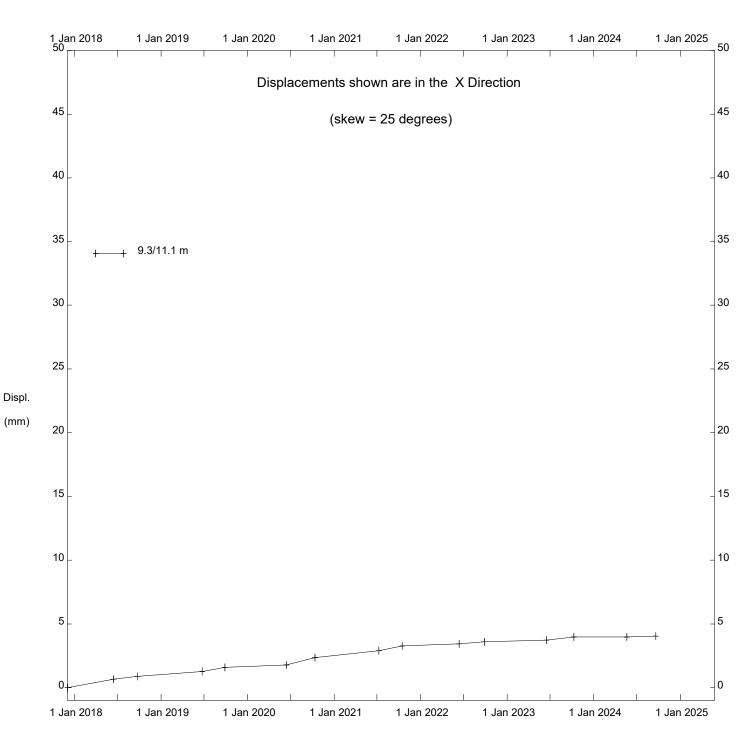




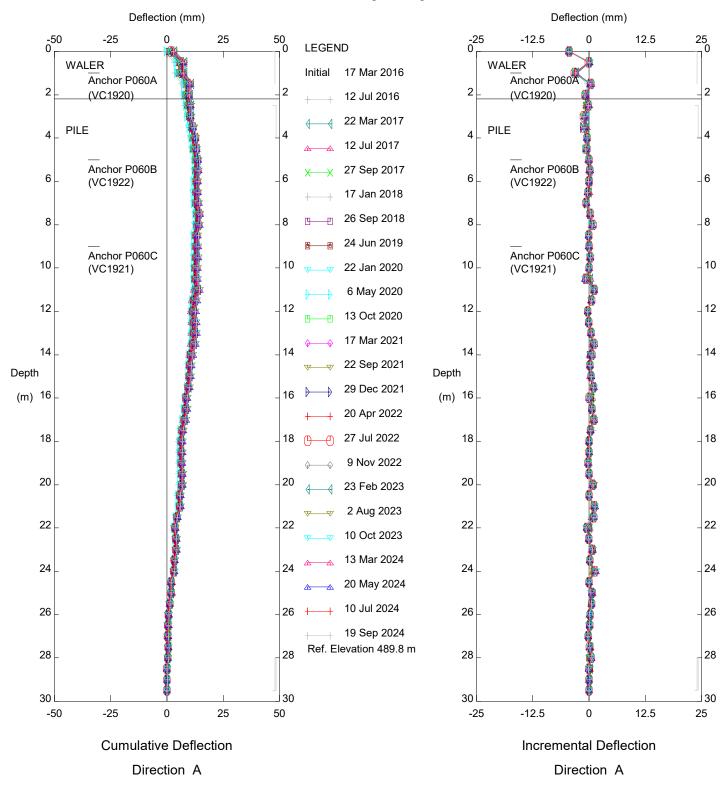
PH071 Daishowa West, Inclinometer SI16-3

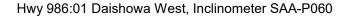


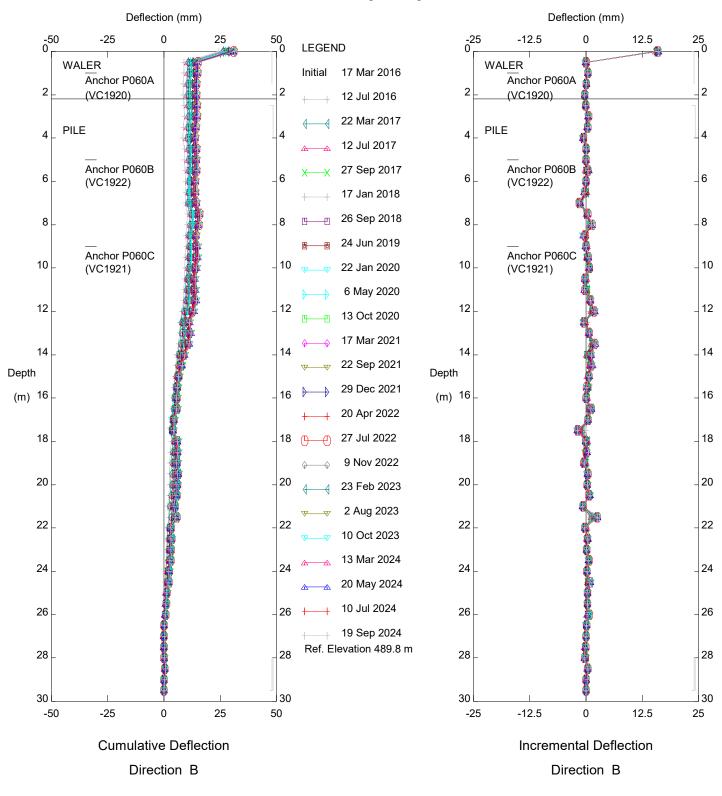




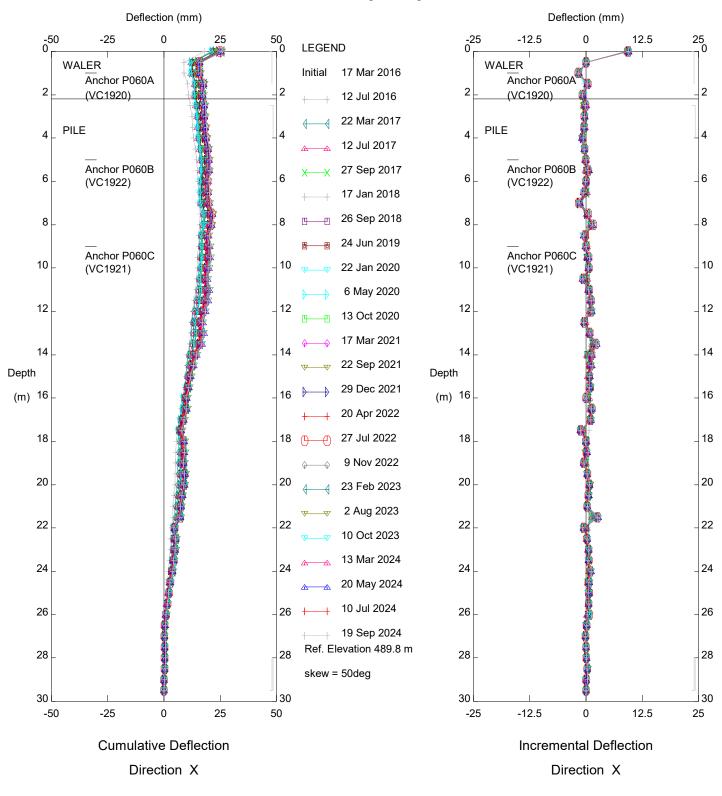
PH071 Daishowa West, Inclinometer SI16-3



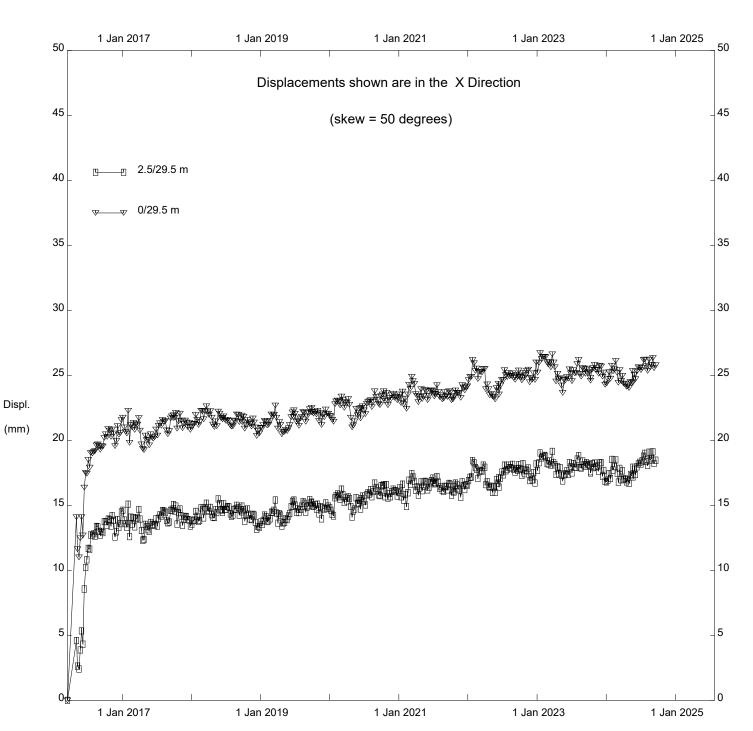




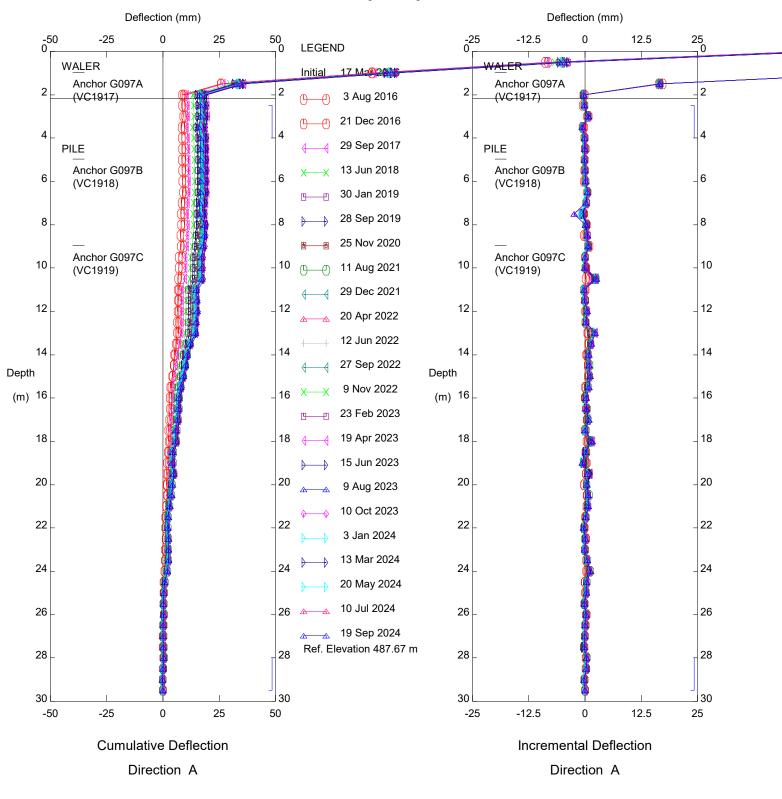




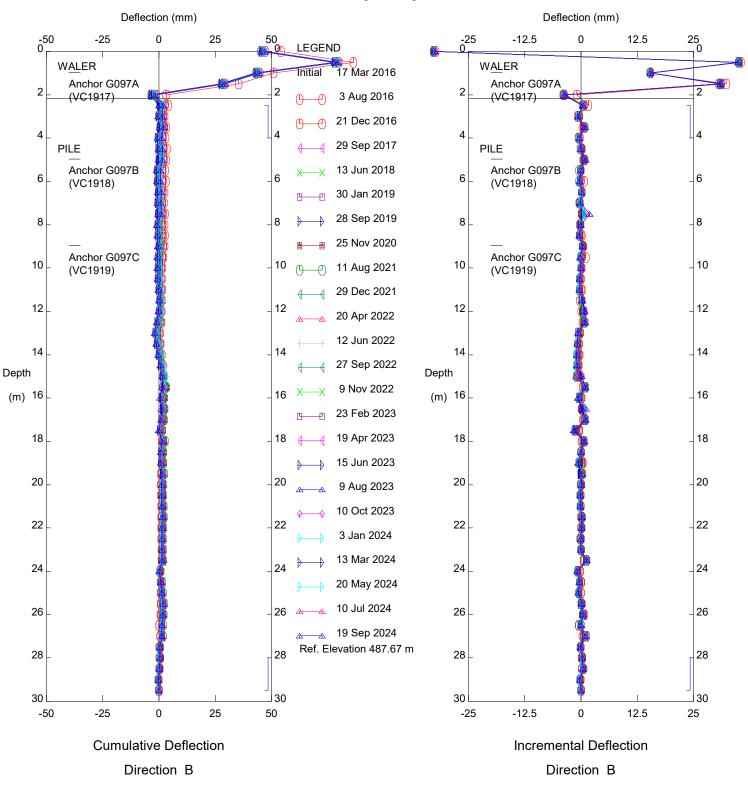


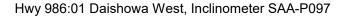


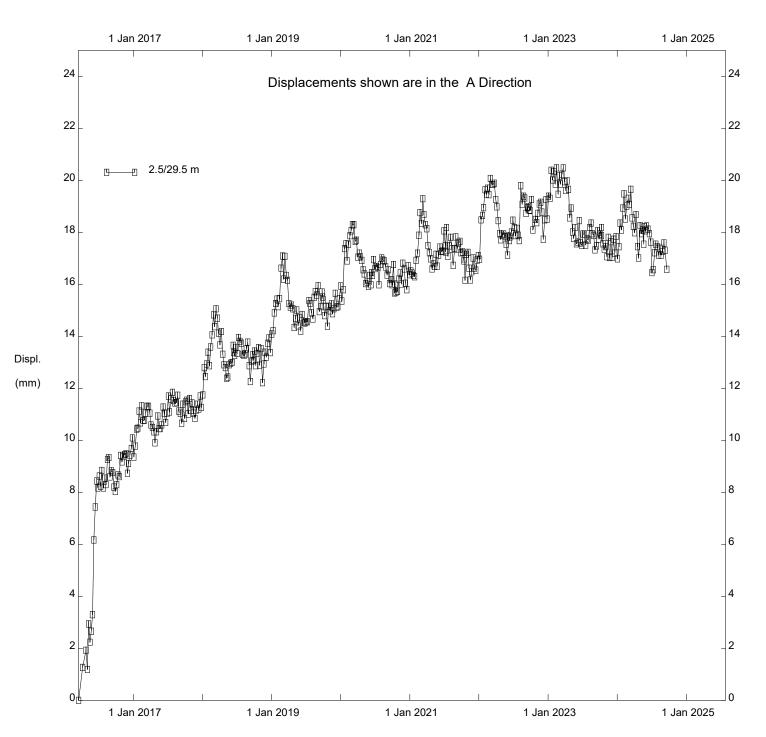
Hwy 986:01 Daishowa West, Inclinometer SAA-P060



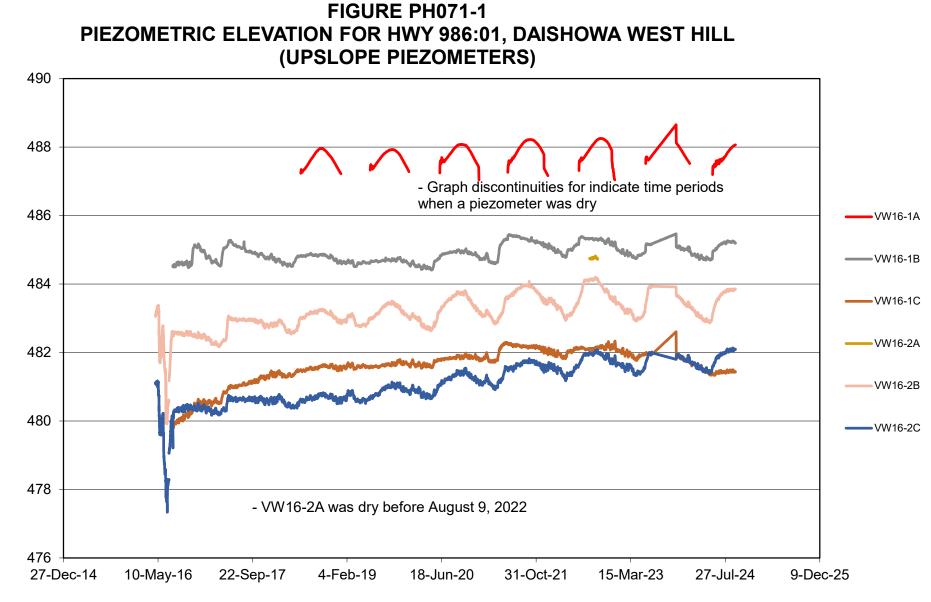




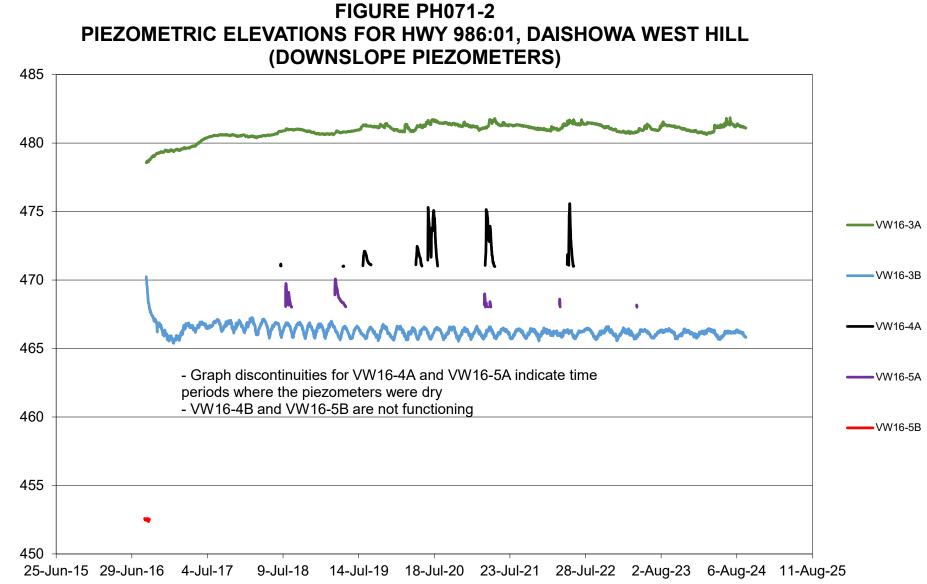




Hwy 986:01 Daishowa West, Inclinometer SAA-P097



DATE

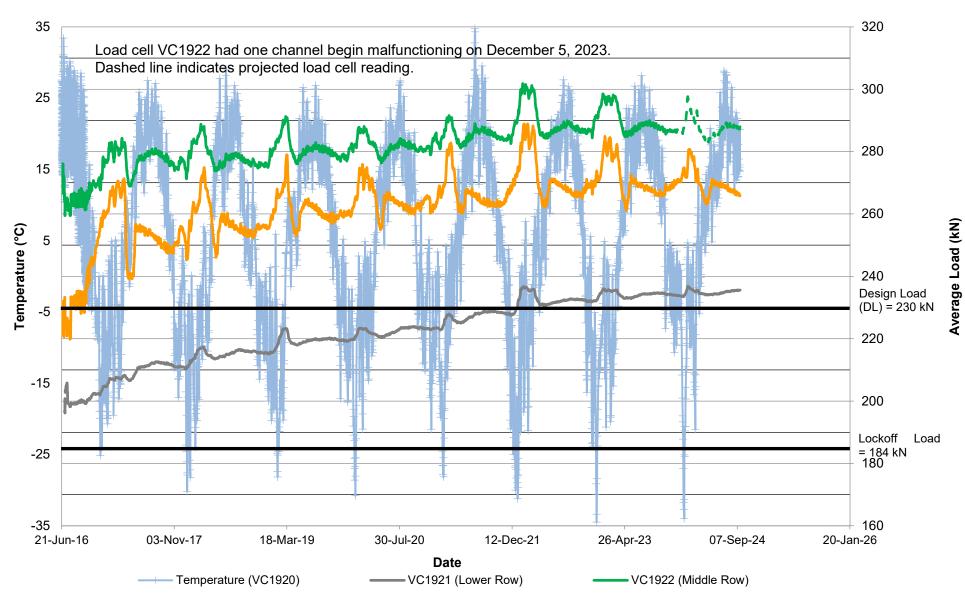


35 300 25 280 15 260 Temperature (°C) 5 240 Design Load (DL) = 230 kN 220 -5 200 -15 Lockoff Load = 184 kN -25 180 Load cell VC1924 had one channel begin malfunctioning on September 16, 2021, load cell VC1923 had one channel begin malfunctioning on October 4, 2021. Dashed lines indicate projected load cell readings. -35 160 21-Jun-16 03-Nov-17 18-Mar-19 30-Jul-20 12-Dec-21 26-Apr-23 07-Sep-24 20-Jan-26 Date —— Temperature (VC1924) 

Average Load (kN)

FIGURE PH071-3 LOAD CELL DATA FOR HWY 986:01, DAISHOWA WEST (PILE P022)

FIGURE PH071-4 LOAD CELL DATA FOR HWY 986:01, DAISHOWA WEST (PILE P060)



35 320 25 300 15 280 Average Load (kN) Temperature (°C) 5 260 -5 240 Design Load (DL) = 230 kN -15 220 -25 200 Load cell VC1918 had one channel begin malfunctioning on May 15, 2022. Dashed line indicates projected load cell reading Lockoff Load = 184 kN -35 180 21-Jun-16 03-Nov-17 18-Mar-19 30-Jul-20 12-Dec-21 26-Apr-23 07-Sep-24 20-Jan-26 Date - Temperature (VC1917) ----VC1918 (Projected)

FIGURE PH071-5 LOAD CELL DATA FOR HWY 986:01, DAISHOWA WEST (PILE P097)