ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS GRMP PEACE REGION – (PEACE RIVER DISTRICT) INSTRUMENTATION MONITORING - SPRING 2024



Site Number	Location	Name	Hwy	km
PH032	HWY 744:04 C1 58.0	Makeout Slide- Judah Hill	744:04	Km 58.0
Legal Description	n: 9-20-83-21 W5	UTM Co-ordinates		
		11U E 483237	N 622	29841

Current Monitoring:	23-May-2024	Previous Monitoring	9-Oct-2023
Instruments Read By:	Mr. Niraj Regm	i, G.I.T and Mr. Nixson Mationg,	of Thurber

Instruments Read During This Site Visit									
Slope Inclinometers (SIs): PK15, PK36, PK54 and PK80 (at KM 58 pile wall) PM12 and PM24 (at Makeout pile wall)	Pneumatic Piezometers (PN): PN13-32-1S and PN13 32 1D	Vibrating Wire Piezometers (VW):	Standpipe Piezometers (SP):						
Load Cell (LC): VC1850, VC1853, VC1855, VC1856, VC1857, VC1858, VC1859, VC1860, VC1861 and VC1862 (on selected anchors at KM 58 pile wall)	Strain Gauges: N/A	SAAs:	Others:						
VC1848, VC1849, VC1851, VC1852 and VC1854 (on selected anchors at Makeout pile wall)									

Readout Equipment Used									
Slope Inclinometers: RST Digital Inclinometer probe with 2 ft wheelbase and RST Pocket PC readout	Pneumatic Piezometers: RST C108 pneumatic piezometer readout	Vibrating Wire Piezometers:	Standpipe Piezometers:						
Load Cell: RST DT2040 datalogger (Load cell datalogger files were uploaded to a laptop using RST Multichannel DTLink software)	Strain Gauges:	SAAs:	Others:						

Discussion								
Zones of New Movement:	None							
Interpretation of Monitoring Results:	At the KM 58 pile wall, PK15 showed a rate of movement of less than 0.1 mm/yr the length of the pile and a rate of movement of 0.1 mm/yr over the combined length of the pile and waler since the fall of 2023 readings. Since the completion of construction, PK15 has shown a total cumulative deflection of 3.0 mm over the length of the pile in the downslope direction and a total cumulative movement of 3.4 mm in the downslope direction over the combined length of the pile and waler. PK36 showed a rate of movement of 0.5 mm/yr over the length of the pile and a rate of movement of 1.9 mm/yr over the combined length of the pile and waler. Since the completion of construction, PK36 has shown total cumulative deflections of 5.3 mm in the downslope direction over the length of the pile and 5.1 mm in the downslope direction over the combined length of the pile and waler.							

PK54 showed a rate of movement of 1.8 mm/yr over the length of the pile and a rate of movement of 1.2 mm/yr over the combined length of the pile and waler. Since the completion of construction, PK54 has shown total cumulative movements of 12.9 mm in the downslope direction over the length of the pile and 10.9 mm in the downslope direction over the combined length of the pile and waler.

PK80 showed a rate of movement of 0.7 mm/yr over the length of the pile and no discernible movement over the combined length of the pile and waler. Since the completion of construction, PK80 has shown total cumulative movements of 9.3 mm of in the downslope direction over the length of the pile and 7.9 mm in the downslope direction over the combined length of the pile and waler.

The SIs at the KM 58 wall location show a current overall trend of slow downslope movement.

At the Makeout pile wall location, PM12 showed a rate of movement of 2.6 mm/yr over the length of the pile and a rate of movement of 5.8 mm/yr over the combined length of the pile and waler. Since the completion of construction, PM12 has shown total cumulative deflections of 2.7 mm in the downslope direction over the length of the pile and 1.5 mm in the upslope direction over the combined length of the pile and waler.

PM24 showed a rate of movement of 1.0 mm/yr over the length of the pile and a rate of movement of 0.7 mm/yr over the combined length of the pile and waler. Since the completion of construction, PM24 has shown total cumulative movements of 3.0 mm in the downslope direction over the length of the pile and 1.3 mm in the downslope direction over the combined length of the pile and waler.

After being pulled into the slope during the initial lock off of the anchors, the SIs at the Makeout wall location have show an overall trend of slow downslope movement since the end of construction. There have been minor seasonal changes in the wall displacement.

PN13-32-1S showed an increase in groundwater level of 0.07 m since it was last read in the spring of 2023 readings. PN13-32-1D showed a decrease in groundwater level of 0.05 m since the fall of 2023 readings. Pneumatic piezometer results are plotted in Figures PH032-1 (by elevation) and PH032-2 (by depth below ground surface) in Appendix A.

The load cells are connected to two dataloggers that are programmed to take two readings per day. Since the fall of 2023 readings, the load cells at the KM 58 wall showed minor changes in measured load ranging from a decrease of 1.18 kN in VC1853 (anchor K54L) to an increase of 6.79 kN in VC1862 (anchor K15M). Load cells VC1862 (K15M), VC1858 (K15L), VC1856 (K36M), VC1857 (K54M), VC1860 (K79M) and VC1861 (K80L) registered all time high measured loads between January 28 to 30, 2024. The anchors at the KM 58 wall show an overall trend of slowly increasing load, mainly with seasonally higher loads during the winter months. It should also be noted that load cells VC1862 (K15M) and VC1858 (K15L) show current loads that are 4.7 percent and 7.9 percent, respectively, above their SLS design loads.

At the Makeout wall, the load cells showed increases in measured load ranging from 0.21 kN in VC1848 (anchor M12L) to 5.01 kN in VC1854 (anchor M12U). The load cells at the Makeout wall have also shown an overall trend of slowly increasing loads since the end of construction, with seasonably higher loads during the winter months. However, none of the measured loads are over the SLS design loads.

The load cell average loads and temperatures are plotted for the KM 58 and Makeout walls on Figures PH032-3 and PH032-4, respectively, in Appendix A. The design and lock-off loads for each anchor are shown in the legends of the figures.

	Overall, the SI and load cell data indicates that the pile walls have been effective at mitigating the landslide movements at this site and the measured deflections and anchor loads are within expected ranges. However, since the instruments at the KM 58 pile wall are showing a trend of downslope movement, combined with gradually increasing anchor loads, the instruments here should be monitored closely to ensure that the downslope movement doesn't begin to accelerate.								
Future Work:	The instruments should be read again in the fall of 2024.								
Instrumentation Repairs:	Pneumatic Piezometer PN13-32-1S was malfunctioning during the fall of 2023 readings, however it was found to be functioning during the current readings, although the readings were slow to stabilize.								
Additional Comments:									
Attachments:	 Table PH032-1: Spring 2024 – HWY 744:04 Judah Hill (Makeout Slide) Slope Inclinometer Instrumentation Reading Summary Table PH032-2: Spring 2024 – HWY 744:04 Judah Hill (Makeout Slide) Pneumatic Piezometer Instrumentation Reading Summary Table PH032-3: Spring 2024 – HWY 744:04 Judah Hill (Makeout Slide) Load Cell Instrumentation Reading Summary Statement of Limitations and Conditions Appendix A Field Inspector's report Site Plan Showing Approximate Instrument Locations (Drawings No. 32121-PH032-1, 32121-PH032-2, and 32121-PH032-3) Pile Wall General Layout drawings SI Reading Plots Figure PH032-1 (Piezometric Elevations) Figure PH032-3 (Load Cell Data for Km 58 Pile Wall) Figure PH032-4 (Load Cell Data for Makeout Pile Wall) 								

We trust this report meets your requirements at present. If you have any questions, please contact the undersigned at your convenience.

Yours very truly, Thurber Engineering Ltd. Don Proudfoot, M.Eng., P. Eng. Partner | Senior Geotechnical Engineer

Bruce Nestor, P.Eng. Geotechnical Engineer



Table PH032-1: Spring 2024 – HWY 744:04 Judah Hill (Makeout Slide) Slope Inclinometer Instrumentation Reading Summary

Date Monitored: May 23, 2024

INSTRUMENT #	DATE INITIALIZED (AFTER CONSTRUCTION)	TOTAL CUMULATIVE RESULTANT MOVEMENT AT NOTED DEPTH SINCE INITIAL READING (mm)	MAXIMUM RATE OF MOVEMENT (mm/yr)	CURRENT STATUS	DATE OF PREVIOUS READING	INCREMENTAL MOVEMENT SINCE PREVIOUS READING (mm)	RATE OF MOVEMENT (mm/yr)	CHANGE IN RATE OF MOVEMENT SINCE PREVIOUS READING (mm/yr)
			KM 58 V	VALL				
PK15	July 2, 2015	3.0 over 2.1 m to 13.7 m depth in 274° direction	17.3 in July 2015	Operational	October 9,	<0.1	<0.1	-2.6
FKIS	July 2, 2015	3.4 over 0.3 m to 13.7 m depth in 274° direction	depth in 274° 29.1 in		2023	0.1	0.1	-0.4
PK36	July 2, 2015	5.3 over 2.6 m to 16.6 m depth in 318° direction	3.4 in October 2020	Operational	October 9,	0.3	0.5	-0.2
FRSO	July 2, 2013	5.1 over 0.1 to 16.6 m depth in 318° direction	8.0 in September 2016	Operational	2023	1.2	1.9	10.3
DK54	luly 2, 2015	20.4 m depth in 313° October		12.0 in October 2020		1.1	1.8	-0.1
PK54	July 2, 2015	10.9 over 0.3 m to 20.4 m depth in 313° direction	13.3 in October 2020	Operational	2023	0.7	1.2	-0.4

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.



Table PH032-1 – Continued...Spring 2024 – HWY 744:04 Judah Hill (Makeout Slide) Slope Inclinometer Instrumentation Reading Summary

Date Monitored: May 23, 2024

	TOTAL						
RE MOVIZED NOT SING	VEMENT AT TED DEPTH CE INITIAL	MAXIMUM RATE OF MOVEMENT (mm/yr)	CURRENT STATUS	DATE OF PREVIOUS READING	INCREMENTAL MOVEMENT SINCE PREVIOUS READING (mm)	RATE OF MOVEMENT (mm/yr)	CHANGE IN RATE OF MOVEMENT SINCE PREVIOUS READING (mm/yr)
20.0 m	n depth in 262°	-20.2 in July 2015	Operational October 9		0.4	0.7	-1.4
20.0 m	n depth in 262°	-26.4 in July 2015	Operational	2023	No discernible movement	N/A	-6.6
		MAKEOU					
19.2 m	n depth in 316°	-41.3 in July 2015	Operational	October 9,	1.6	2.6	4.1
1.5 0 19.2 m	n depth in 316°	-52.8 in July 2015	Operational	2023	3.6	5.8	11.4
19.2 m	n depth in 298°	-27.4 in July 2015	Operational	October 9,	0.9	1.0	0.1
19.2 m	n depth in 298° direction	-33.4 in July 2015		2023	0.6	0.7	0.1
	2015 P.2 m	2015 NOTED DEPTH SINCE INITIAL READING (mm) 9.3 over 2.4 m to 20.0 m depth in 262° direction 7.9 over 0.5 m to 20.0 m depth in 262° direction 2.7 over 2.2 m to 19.2 m depth in 316° direction 1.5 over 0.3 m to 19.2 m depth in 316° direction 3.0 over 2.1 m to 19.2 m depth in 298° direction 1.3 over 0.3 m to 19.2 m depth in 298° direction 1.3 over 0.3 m to 19.2 m depth in 298° direction	## Action	MOVEMENT AT NOTED DEPTH SINCE INITIAL READING (mm) 9.3 over 2.4 m to 20.0 m depth in 262° direction 7.9 over 0.5 m to 20.0 m depth in 262° direction 7.9 over 0.5 m to 20.0 m depth in 262° direction 26.4 in July 2015	MOVEMENT AT NOTED DEPTH SINCE INITIAL READING (mm)	MOVEMENT AT NOTED DEPTH SINCE INITIAL READING (mm) PREVIOUS READING (mm)	Note Depth SINCE Initial Reading (mm) STATUS PREVIOUS READING (mm) (mm/yr) STATUS PREVIOUS READING (mm) (mm/yr) (m

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.



Table PH032-2: Spring 2024 – HWY 744:04 Judah Hill (Makeout Slide) Pneumatic Piezometer Instrumentation Reading Summary

Date Monitored: May 23, 2024

INSTRUMENT #	DATE INITIALIZED	TIP DEPTH (m)	GROUND ELEV. (m)	CURRENT STATUS	HIGHEST MEASURED WATER LEVEL (m)	MEASURED PORE PRESSURE (kPa)	CURRENT GROUNDWATER ELEVATION (m)	PREVIOUS GROUNDWATER ELEVATION October 9, 2023 (m)	CHANGE IN WATER LEVEL SINCE PREVIOUS READING (m)
PN13-32-1S	November 30, 2013	9.14	499.84	Operational	493.56 in September 2022	23.6	493.10	493.03*	0.07
PN13-32-1D	November 30, 2013	18.29	499.84	Operational	482.46 in December 2013	4.6	482.02	482.07	-0.05

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site. PN 13-32-1S was not functioning properly in fall 2023 so the previous reading presented was taken on June 16, 2023



Table PH032-3: Spring 2024 – HWY 744:04 Judah Hill (Makeout Slide) Load Cell Instrumentation Reading Summary

Date Monitored: May 23, 2024

ANCHOR NUMBER	LOAD CELL SERIAL #	DAD CELL DESIGN LOAD / RECORDED LOAD / LOAD (MAY :		RECORDED LOAD ⁽¹⁾ (MAY 23, 2024) (kN)	PREVIOUS RECORDED LOAD (1) (OCT. 9, 2023) (kN)	CHANGE IN LOAD SINCE PREVIOUS READING (kN)
			KM 58 WALL			
K15M	VC1862	178/177	194.23 on January 30, 2024	186.31	179.52	6.79
K15L	VC1858	239/231	264.90 on January 28, 2024	257.87	255.96	1.91
K36M	VC1856	233/199	214.91 on January 30, 2024	200.06	195.11	4.95
K45L	VC1855	292/248	248.50 on April 20, 2015	222.31	222.44	-0.13
K54M	VC1857	231/215	198.10 on January 30, 2024	194.49	193.52	0.97
K54L	VC1853	292/248	240.57 on August 14, 2023	236.67	237.85	-1.18
K55U	VC1850	274/272	275.28 on April 17, 2015	245.24	241.45	3.79
K79U	VC1859	274/272	250.27 on April 16, 2015	217.06	216.93	0.13
K79M	VC1860	231/215	217.55 on January 30, 2024	205.10	202.80	2.30
K80L	VC1861	292/248	264.09 on January 28, 2024	257.42	256.82	0.60

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.

⁽¹⁾ Load cell data is recorded twice daily with datalogger on site. Dataloggers' data are uploaded twice annually during instrumentation readings. See Figure PH032-3 for combined historical instrument readings.



Table PH032-3 - Continued...Spring 2024 - HWY 744:04 Judah Hill (Makeout Slide) Load Cells Instrumentation Reading Summary

Date Monitored: May 23, 2024

ANCHOR NUMBER	LOAD CELL SERIAL #	LOCK-OFF LOAD		RECORDED LOAD ⁽¹⁾ (MAY 23, 2024) (kN)	PREVIOUS RECORDED LOAD (1) (OCT. 9, 2023) (kN)	CHANGE IN LOAD SINCE PREVIOUS READING (kN)
		N	IAKEOUT WALL			
M12U	VC1854	274/272	277.02 on March 18, 2022	253.91	248.90	5.01
M12M	VC1849	231/215	213.90 on March 25, 2015	200.74	199.19	1.55
M12L	VC1848	292/248	253.28 on March 22, 2023	244.15	243.94	0.21
M24U	VC1851	274/272	271.81 on March 25, 2015	248.71	244.75	3.96
M24M	VC1852	231/215	217.10 on March 25, 2015	185.60	182.48	3.12

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.

⁽¹⁾ Load cell data is recorded twice daily with datalogger on site. Dataloggers data are uploaded twice annually during instrumentation readings. See Figure PH032-4 for combined historical instrument readings.



STATEMENT OF LIMITATIONS AND CONDITIONS

1. STANDARD OF CARE

This Report has been prepared in accordance with generally accepted engineering or environmental consulting practices in the applicable jurisdiction. No other warranty, expressed or implied, is intended or made.

2. COMPLETE REPORT

All documents, records, data and files, whether electronic or otherwise, generated as part of this assignment are a part of the Report, which is of a summary nature and is not intended to stand alone without reference to the instructions given to Thurber by the Client, communications between Thurber and the Client, and any other reports, proposals or documents prepared by Thurber for the Client relative to the specific site described herein, all of which together constitute the Report.

IN ORDER TO PROPERLY UNDERSTAND THE SUGGESTIONS, RECOMMENDATIONS AND OPINIONS EXPRESSED HEREIN, REFERENCE MUST BE MADE TO THE WHOLE OF THE REPORT. THURBER IS NOT RESPONSIBLE FOR USE BY ANY PARTY OF PORTIONS OF THE REPORT WITHOUT REFERENCE TO THE WHOLE REPORT.

3. BASIS OF REPORT

The Report has been prepared for the specific site, development, design objectives and purposes that were described to Thurber by the Client. The applicability and reliability of any of the findings, recommendations, suggestions, or opinions expressed in the Report, subject to the limitations provided herein, are only valid to the extent that the Report expressly addresses proposed development, design objectives and purposes, and then only to the extent that there has been no material alteration to or variation from any of the said descriptions provided to Thurber, unless Thurber is specifically requested by the Client to review and revise the Report in light of such alteration or variation.

4. USE OF THE REPORT

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5. INTERPRETATION OF THE REPORT

- a) Nature and Exactness of Soil and Contaminant Description: Classification and identification of soils, rocks, geological units, contaminant materials and quantities have been based on investigations performed in accordance with the standards set out in Paragraph 1. Classification and identification of these factors are judgmental in nature. Comprehensive sampling and testing programs implemented with the appropriate equipment by experienced personnel may fail to locate some conditions. All investigations utilizing the standards of Paragraph 1 will involve an inherent risk that some conditions will not be detected and all documents or records summarizing such investigations will be based on assumptions of what exists between the actual points sampled. Actual conditions may vary significantly between the points investigated and the Client and all other persons making use of such documents or records with our express written consent should be aware of this risk and the Report is delivered subject to the express condition that such risk is accepted by the Client and such other persons. Some conditions are subject to change over time and those making use of the Report should be aware of this possibility and understand that the Report only presents the conditions at the sampled points at the time of sampling. If special concerns exist, or the Client has special considerations or requirements, the Client should disclose them so that additional or special investigations may be undertaken which would not otherwise be within the scope of investigations made for the purposes of the Report.
- b) Reliance on Provided Information: The evaluation and conclusions contained in the Report have been prepared on the basis of conditions in evidence at the time of site inspections and on the basis of information provided to Thurber. Thurber has relied in good faith upon representations, information and instructions provided by the Client and others concerning the site. Accordingly, Thurber does not accept responsibility for any deficiency, misstatement or inaccuracy contained in the Report as a result of misstatements, omissions, misrepresentations, or fraudulent acts of the Client or other persons providing information relied on by Thurber. Thurber is entitled to rely on such representations, information and instructions and is not required to carry out investigations to determine the truth or accuracy of such representations, information and instructions.
- c) Design Services: The Report may form part of design and construction documents for information purposes even though it may have been issued prior to final design being completed. Thurber should be retained to review final design, project plans and related documents prior to construction to confirm that they are consistent with the intent of the Report. Any differences that may exist between the Report's recommendations and the final design detailed in the contract documents should be reported to Thurber immediately so that Thurber can address potential conflicts.
- d) Construction Services: During construction Thurber should be retained to provide field reviews. Field reviews consist of performing sufficient and timely observations of encountered conditions in order to confirm and document that the site conditions do not materially differ from those interpreted conditions considered in the preparation of the report. Adequate field reviews are necessary for Thurber to provide letters of assurance, in accordance with the requirements of many regulatory authorities.

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Geotechnical engineering and environmental consulting projects often have the potential to encounter pollutants or hazardous substances and the potential to cause the escape, release or dispersal of those substances. Thurber shall have no liability to the Client under any circumstances, for the escape, release or dispersal of pollutants or hazardous substances, unless such pollutants or hazardous substances have been specifically and accurately identified to Thurber by the Client prior to the commencement of Thurber's professional services.

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The information, interpretations and conclusions in the Report are based on Thurber's interpretation of conditions revealed through limited investigation conducted within a defined scope of services. Thurber does not accept responsibility for independent conclusions, interpretations, interpretations and/or decisions of the Client, or others who may come into possession of the Report, or any part thereof, which may be based on information contained in the Report. This restriction of liability includes but is not limited to decisions made to develop, purchase or sell land.



ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS GRMP (CON0022164) PEACE REGION (PEACE RIVER DISTRICT) INSTRUMENTATION MONITORING RESULTS

SPRING 2024

APPENDIX A
DATA PRESENTATION

SITE PH032: HWY 744:04, JUDAH HILL (MAKEOUT SLIDE)

ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS PEACE REGION (PEACE RIVER DISTRICT) INSTRUMENTATION MONITORING FIELD SUMMARY (PH032) SPRING 2024

Location: Makeout Slide - Judah Hill (HWY 744:04 C1 57.924) Readout: RST PN C108 Unit 4

File Number: 32121

Probe: RST SET 5R

Cable: RST SET 5R

Read by: NKR/NRM

SLOPE INCLINOMETER (SI) READINGS

SI#	GPS I	Location	Date	Stickup	Depth from top	Magn. North	Cu	Current Bottom		Probe/	Size	Remarks	
	(UT	M 11)		(m)	of Casing (ft)	A+ Groove	Dej	pth Read	lings		Reel	(")	
	Easting (m)	Northing (m)					A+	A-	B+	B-	#		
PK15	483237	6229841	23-May-24	1.21	48 to 2	245	382	-373	545	-549	5R/5R	2.75	
PK36	483225	6229863	23-May-24	0.8	56 to 2	310	-202	213	-39	23	5R/5R	2.75	
PK54	483214	6229882	23-May-24	1.2	70 to 2	300	705	-696	-159	146	5R/5R	2.75	
PK80	483199	6229909	23-May-24	0.99	68 to 2	225	-411	421	218	-238	5R/5R	2.75	
PM12	483157	6229989	23-May-24	1.18	66 to 2	275	-849	856	832	-854	5R/5R	2.75	
PM24	483151	6230002	23-May-24	1.22	66 to 2	260	498	-487	504	-523	5R/5R	2.75	

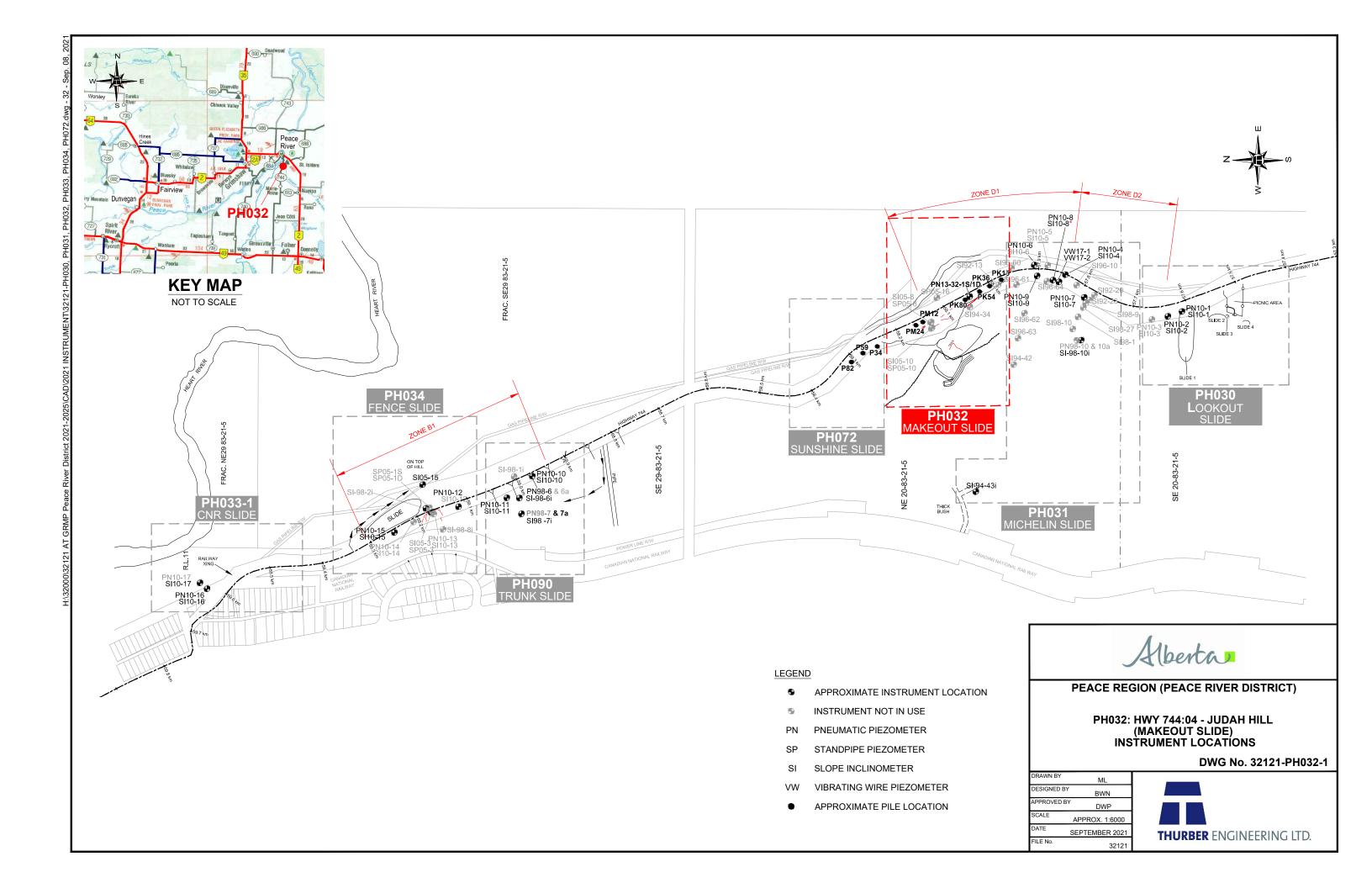
PN#	GPS Location (NAD83)		Date	Reading	Identification
	Easting (m)	Northing (m)		(kPa)	Number
PN13-32-1S	483205	6229901	23-May-24	23.6	35485
PN13-32-1D	483205	6229901	23-May-24	4.6	35497

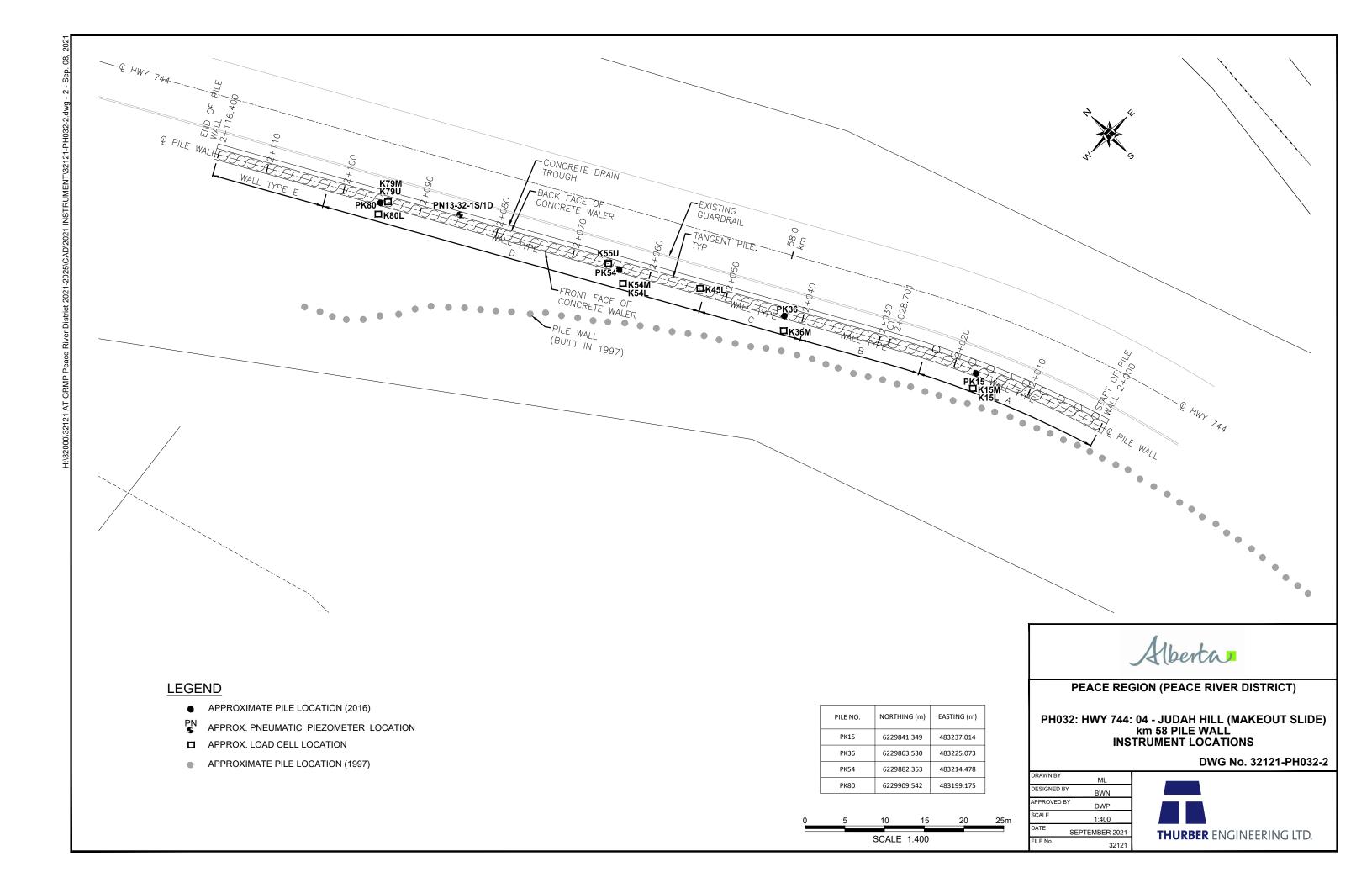
VIBRATING WIRE LOAD CELL (VC) READINGS

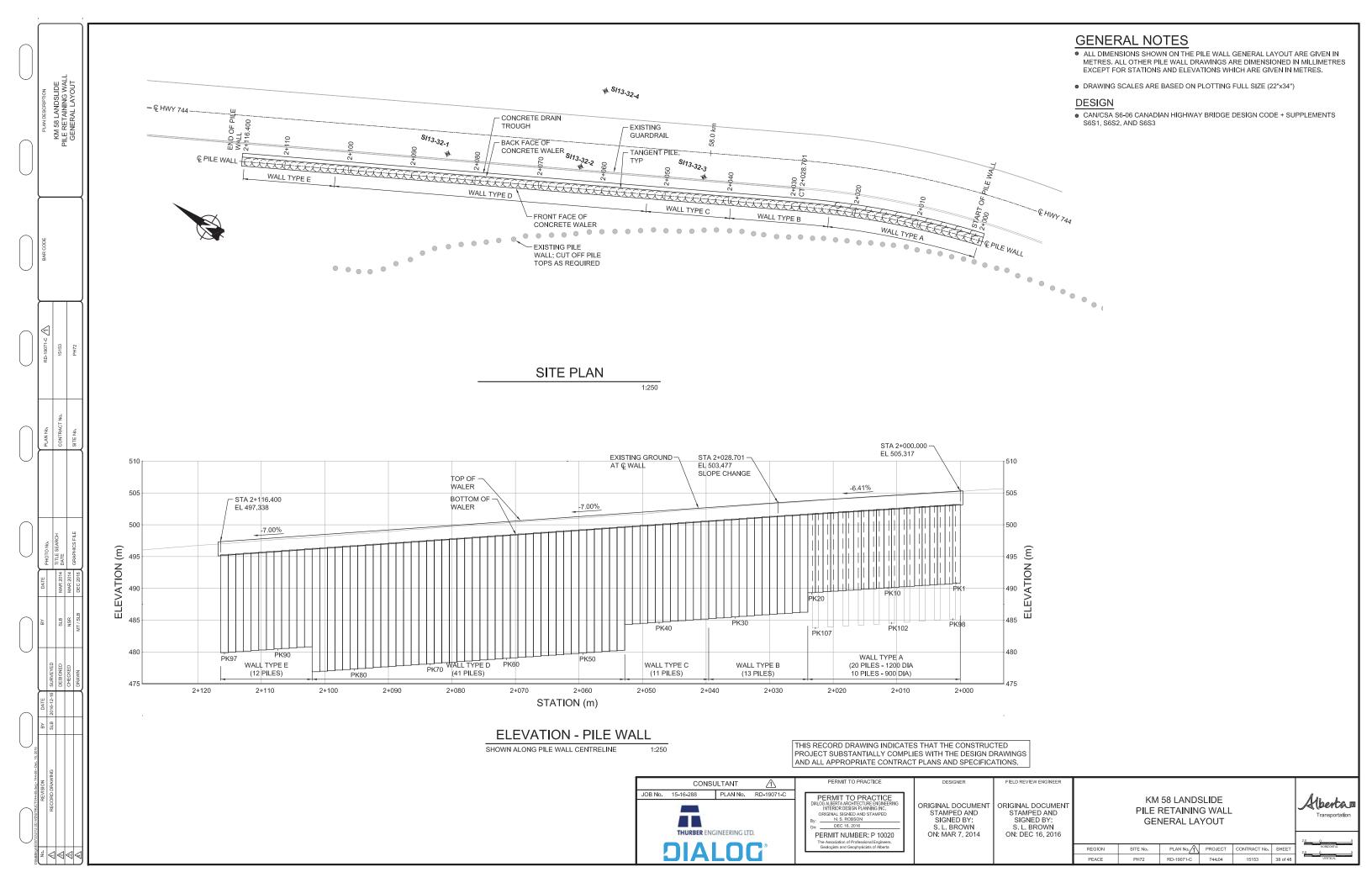
VC#	GPS Location (UTM 11)		Datalogger	Date	
	Easting (m)	Northing (m)	Serial #		Comment
VC1850					Downloaded
VC1853					Downloaded
VC1855					Downloaded
VC1856					Downloaded
VC1857			RST 2034		Downloaded
VC1858				Downloaded	
VC1859					Downloaded
VC1860				23-May-24	Downloaded
VC1861					Downloaded
VC1862					Downloaded
VC1848					Downloaded
VC1849					Downloaded
VC1851			RST 2036		Downloaded
VC1852			1		Downloaded
VC1854					Downloaded

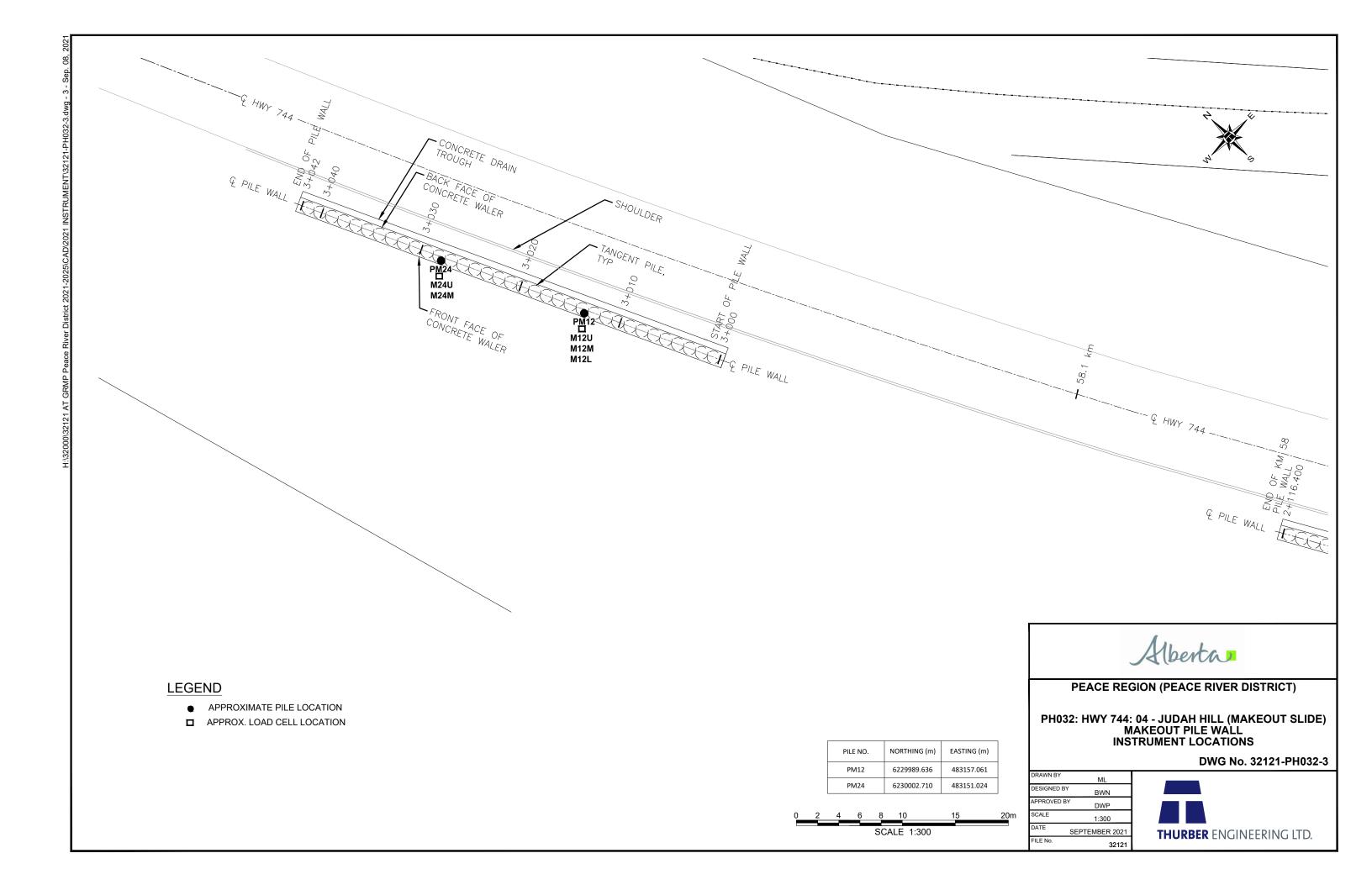
INSPECTOR REPORT

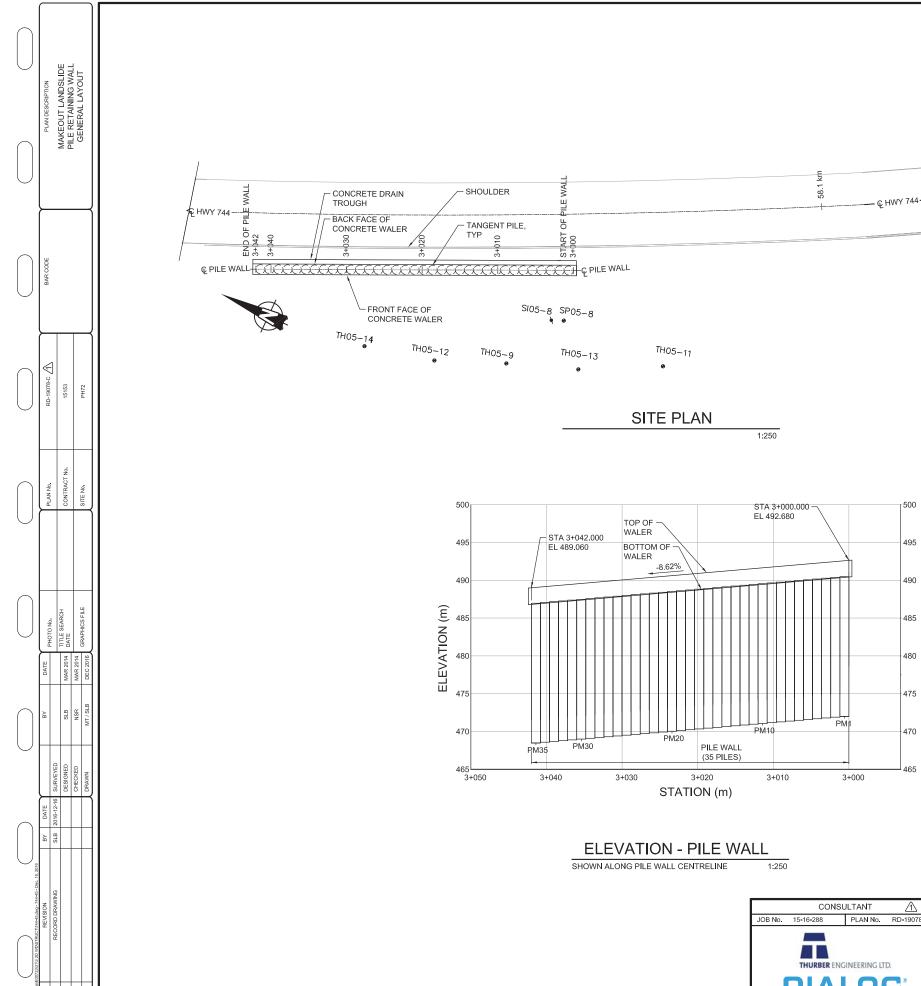
PN 13-32-1S Reading takes a long time to stabilize











GENERAL NOTES

- ALL DIMENSIONS SHOWN ON THE PILE WALL GENERAL LAYOUT ARE GIVEN IN METRES. ALL OTHER PILE WALL DRAWINGS ARE DIMENSIONED IN MILLIMETRES EXCEPT FOR STATIONS AND ELEVATIONS WHICH ARE GIVEN IN METRES.
- DRAWING SCALES ARE BASED ON PLOTTING FULL SIZE (22"x34")

DESIGN

• CAN/CSA S6-06 CANADIAN HIGHWAY BRIDGE DESIGN CODE + SUPPLEMENTS S6S1, S6S2, AND S6S3

REINFORCING STEEL	PLAIN	kg	87 570	-	
CONCRETE - CLASS (m ³	100	-		
CONCRETE - CLASS PILE		m ³	730	-	
DRILLED CONCRETE	DRILL RIG SET-UP	PILE	35	-	
PILES	PILE INSTALLATION	m	644	-	
ITEM	UNIT	TOT EST	AS CONST		
QUANTITY ESTIMATE					



PERMIT TO PRACTICE
DIALOG ALBERTA ARCHITECTURE ENGINEERIN
INTERIOR DESIGN PLANNING INC,
ORIGINAL SIGNED AND STAMPED
BY:
N. S. ROBSON
DEC. 18, 2016

THIS RECORD DRAWING INDICATES THAT THE CONSTRUCTED

PROJECT SUBSTANTIALLY COMPLIES WITH THE DESIGN DRAWINGS AND ALL APPROPRIATE CONTRACT PLANS AND SPECIFICATIONS.

EVATION

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© PILE WALL

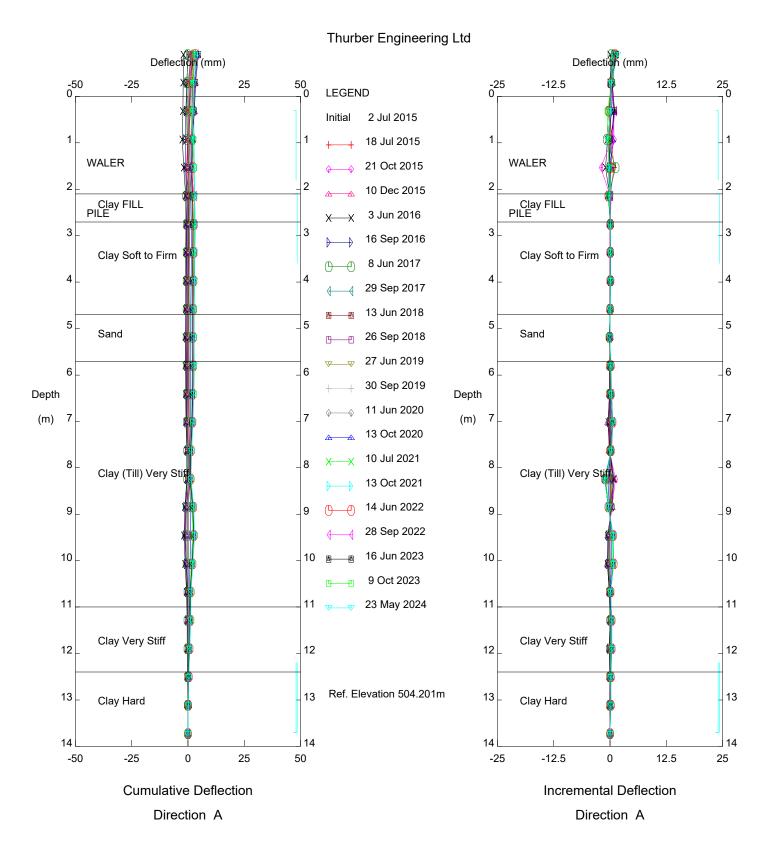
PERMIT NUMBER: P 10020

RIGINAL DOCUMENT ORIGINAL DOCUMENT STAMPED AND SIGNED BY: S. L. BROWN STAMPED AND SIGNED BY: S. L. BROWN ON: MAR 25, 2014 ON: DEC 16, 2016

MAKEOUT LANDSLIDE Albertan PILE RETAINING WALL GENERAL LAYOUT

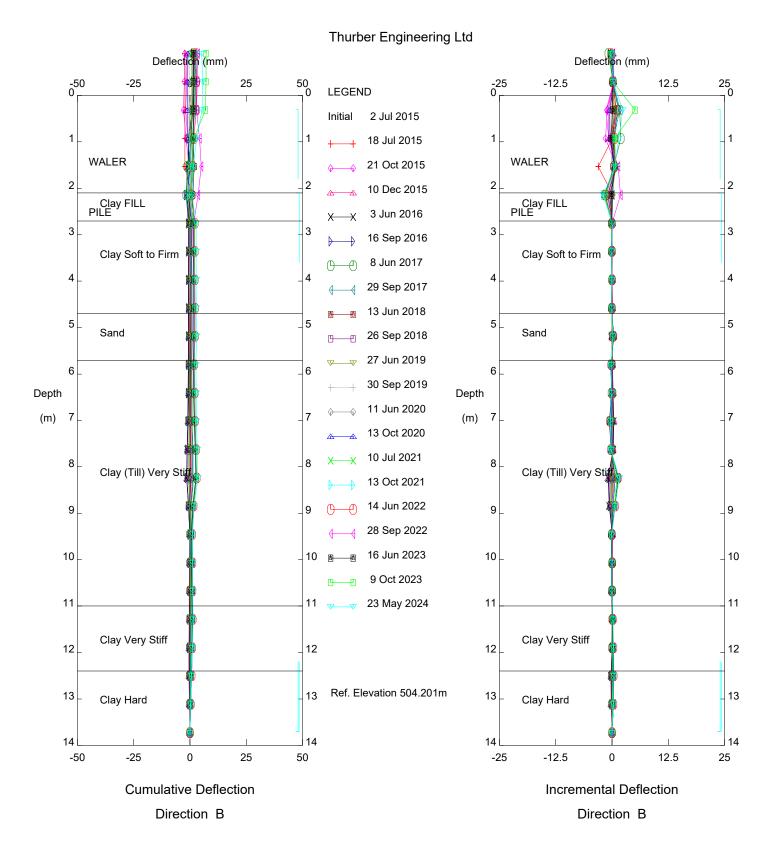
SITE No. PLAN No. PROJECT CONTRACT No. SHEET





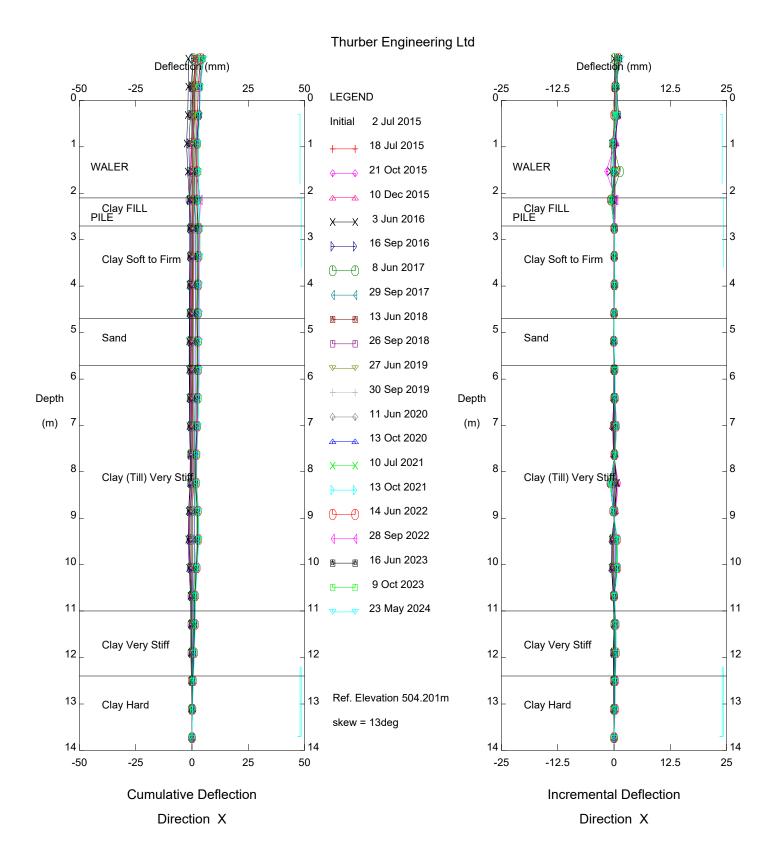
PH032 KM 58 (Post Construction), Inclinometer PK15

Alberta Transportation



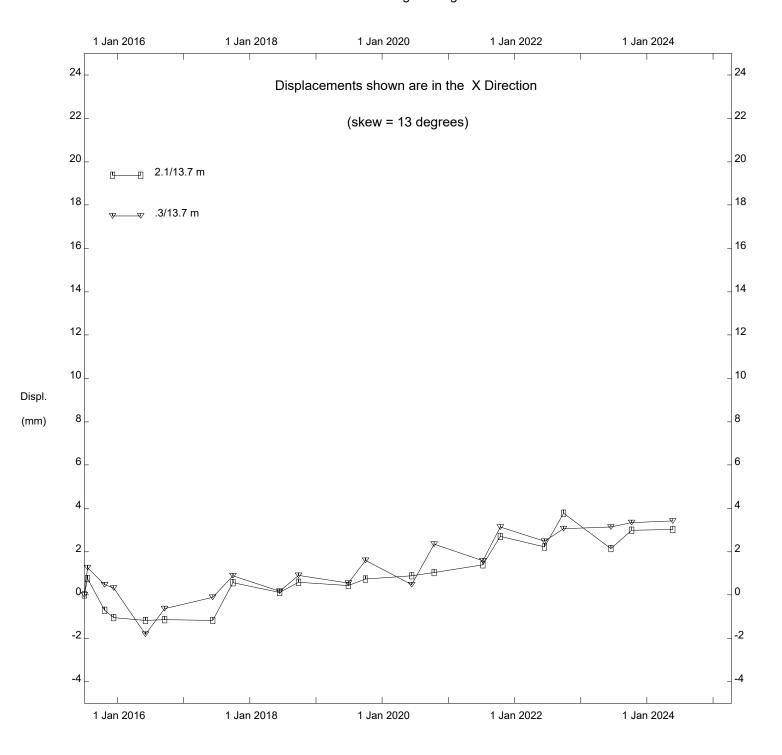
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Alberta Transportation

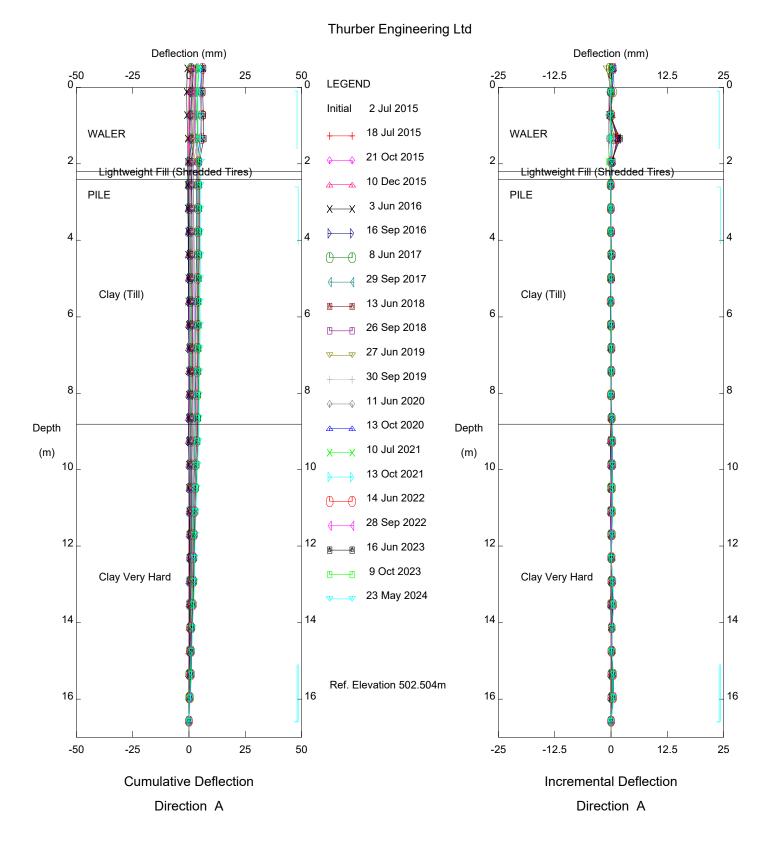


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Alberta Transportation

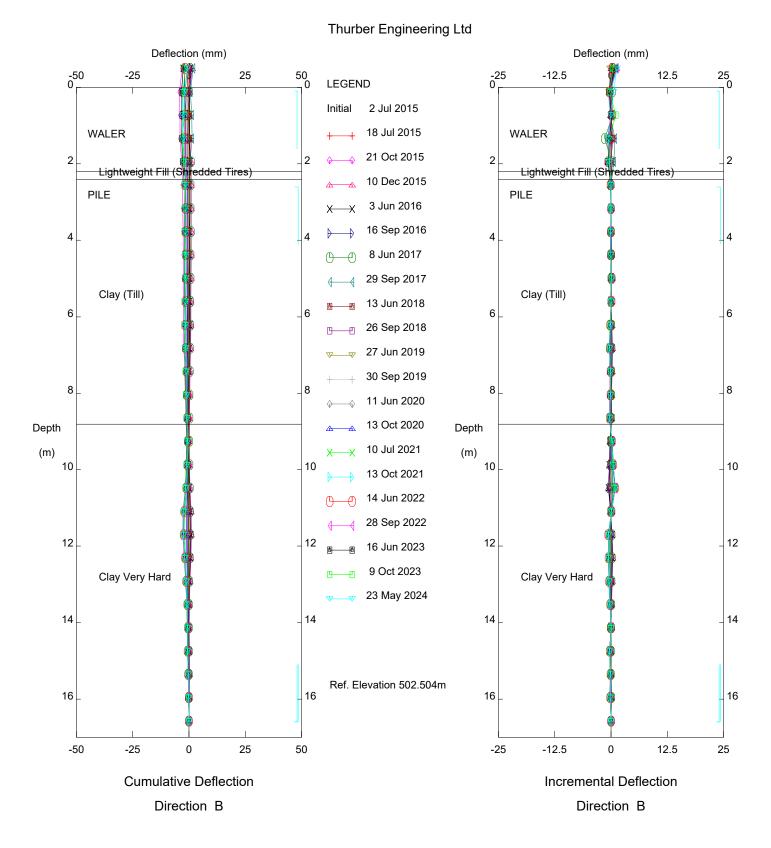


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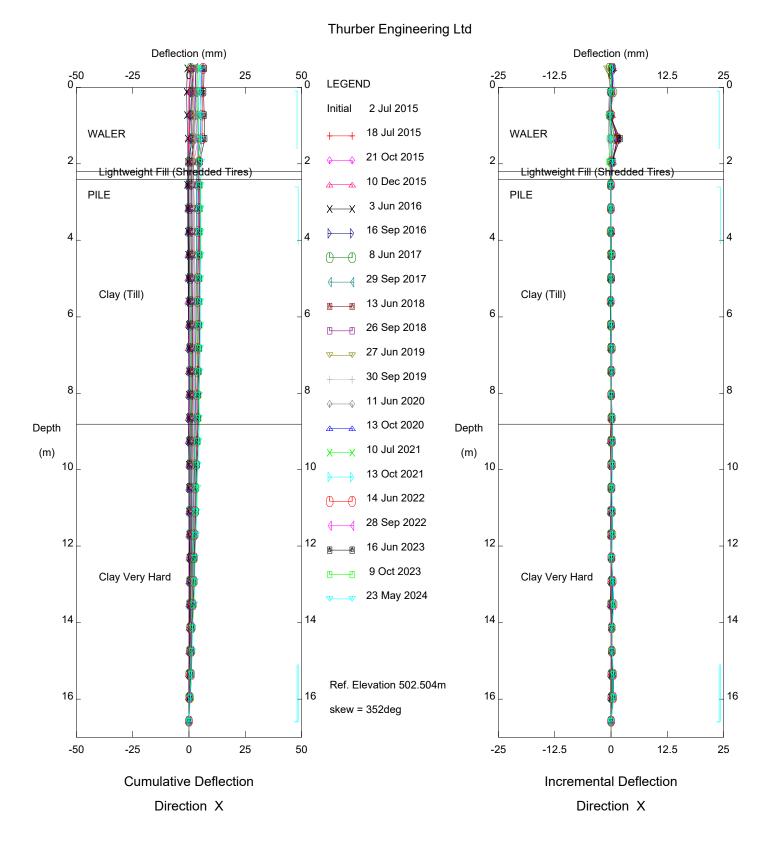
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Alberta Transportation



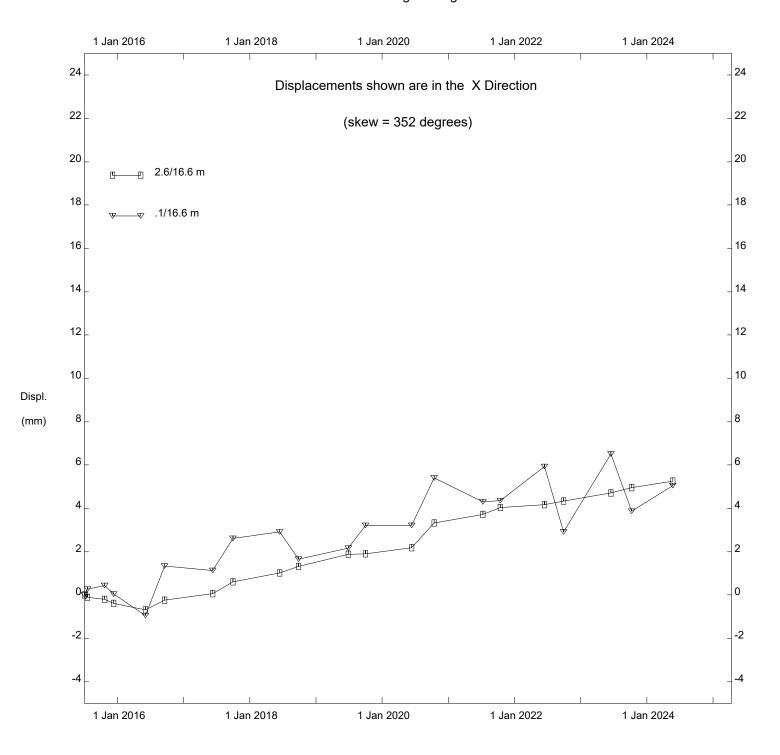
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Alberta Transportation

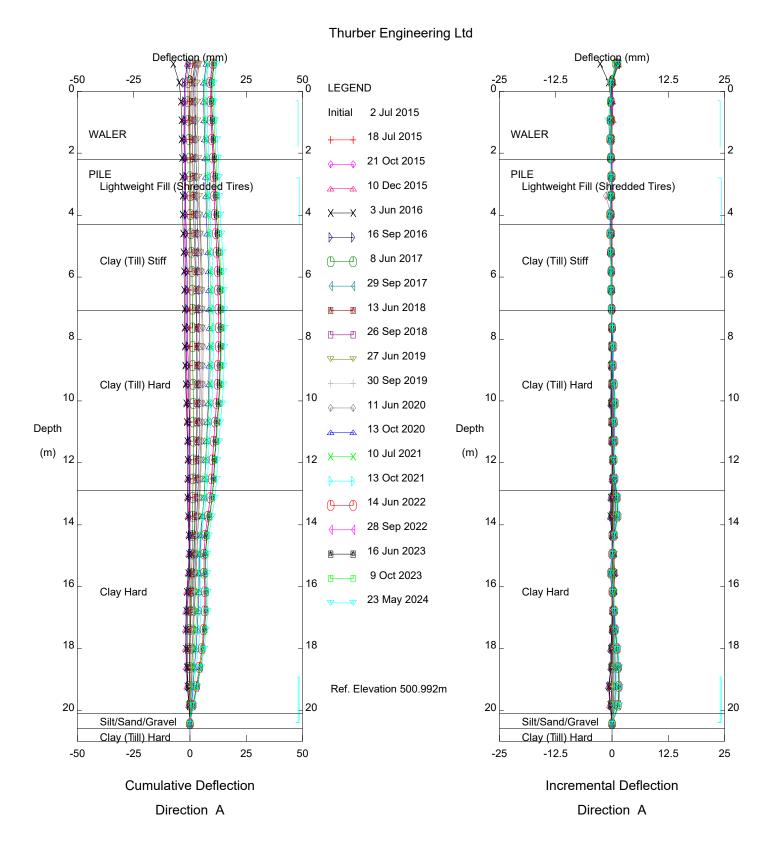


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Alberta Transportation

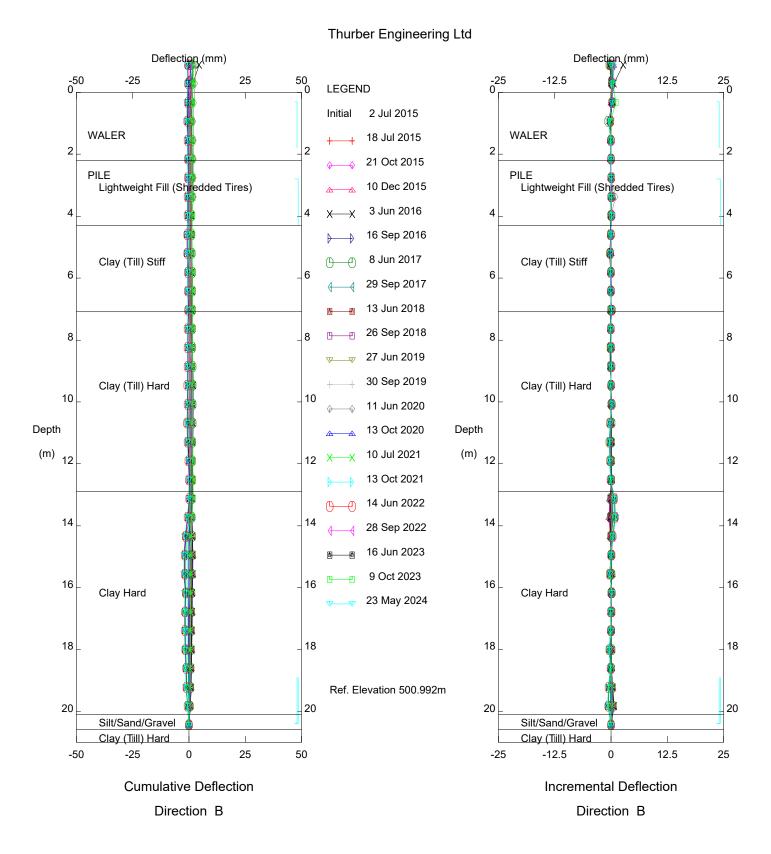


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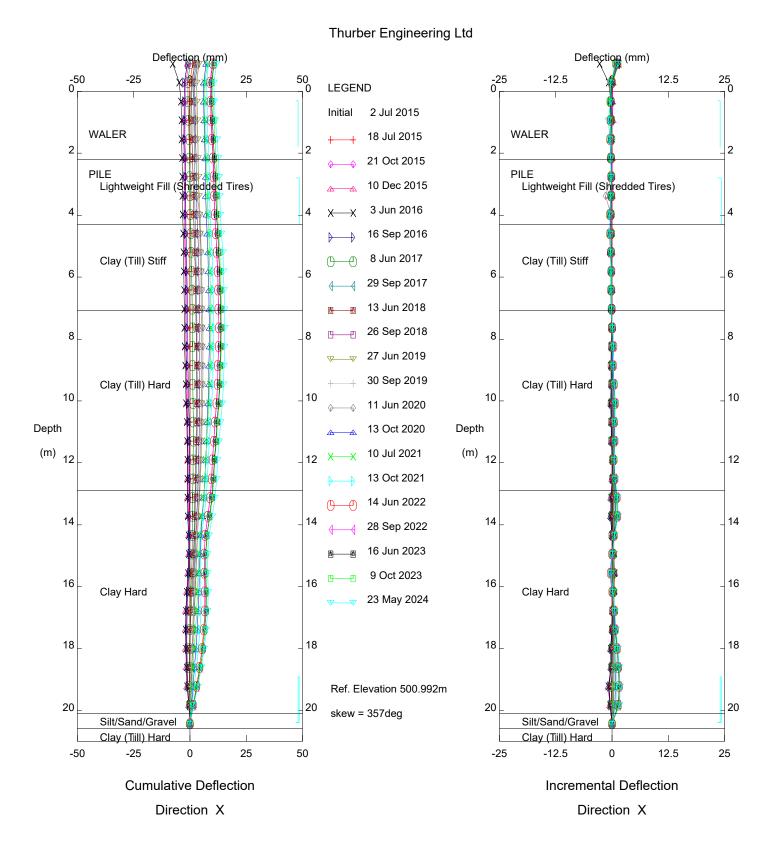
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Alberta Transportation



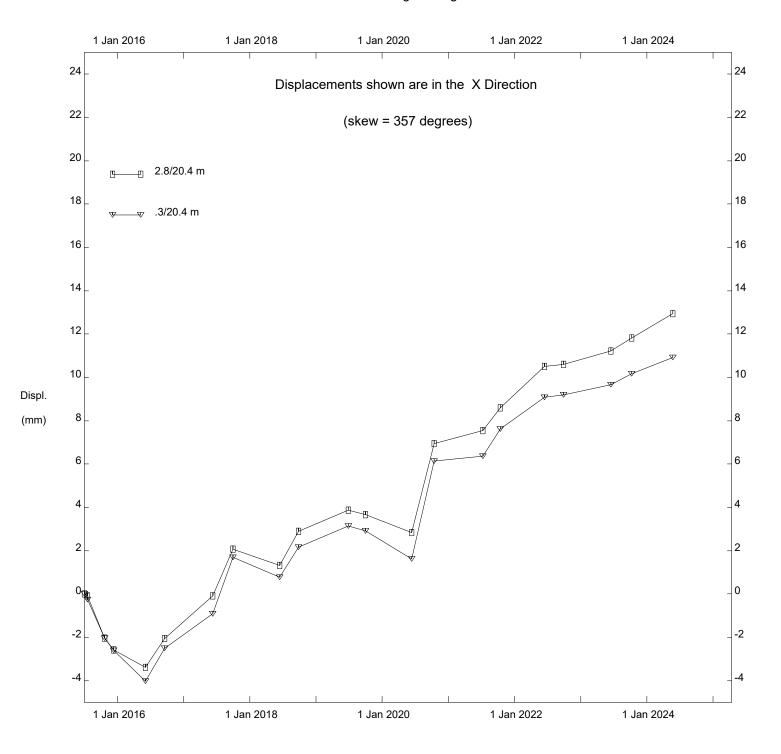
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Alberta Transportation

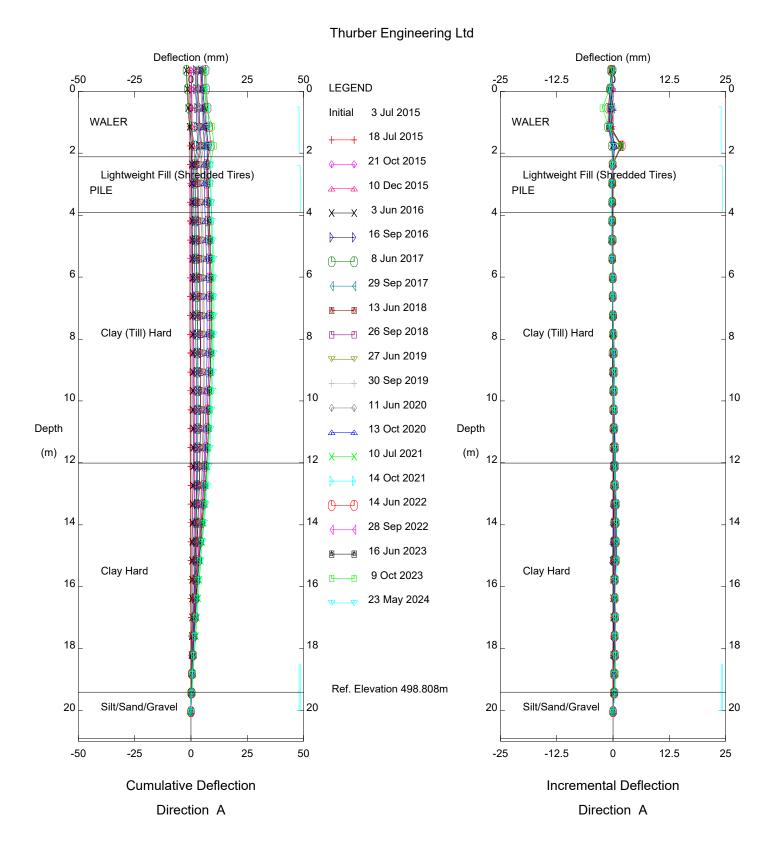


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Alberta Transportation

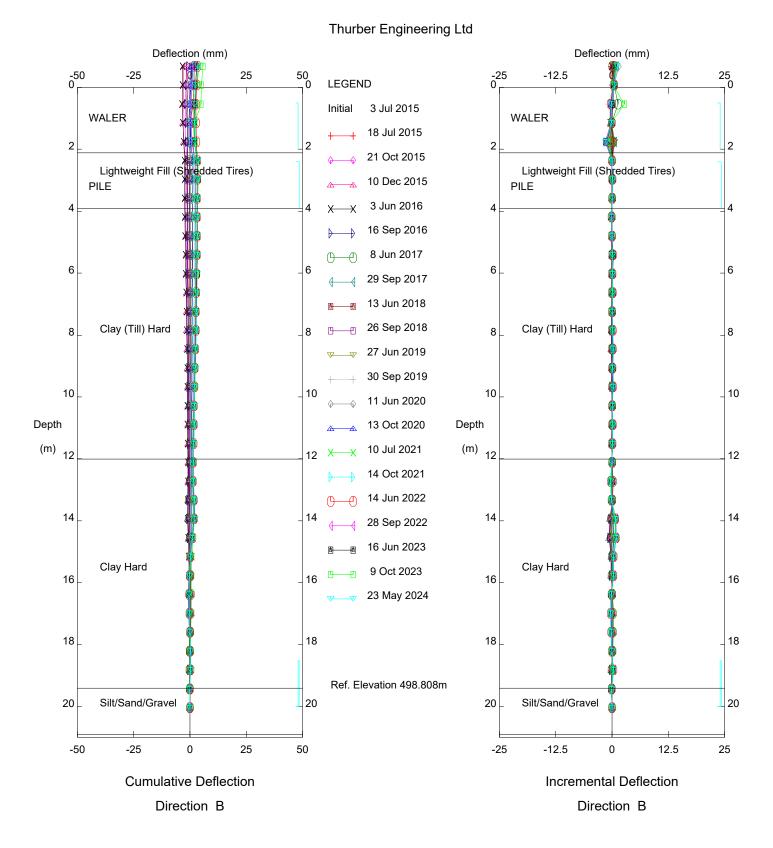


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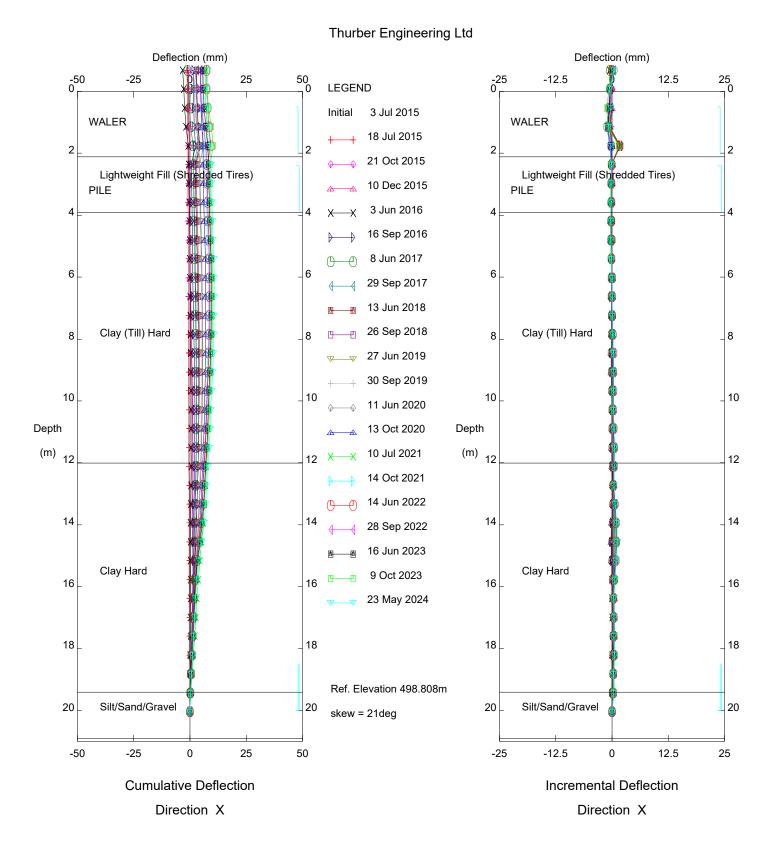
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Alberta Transportation



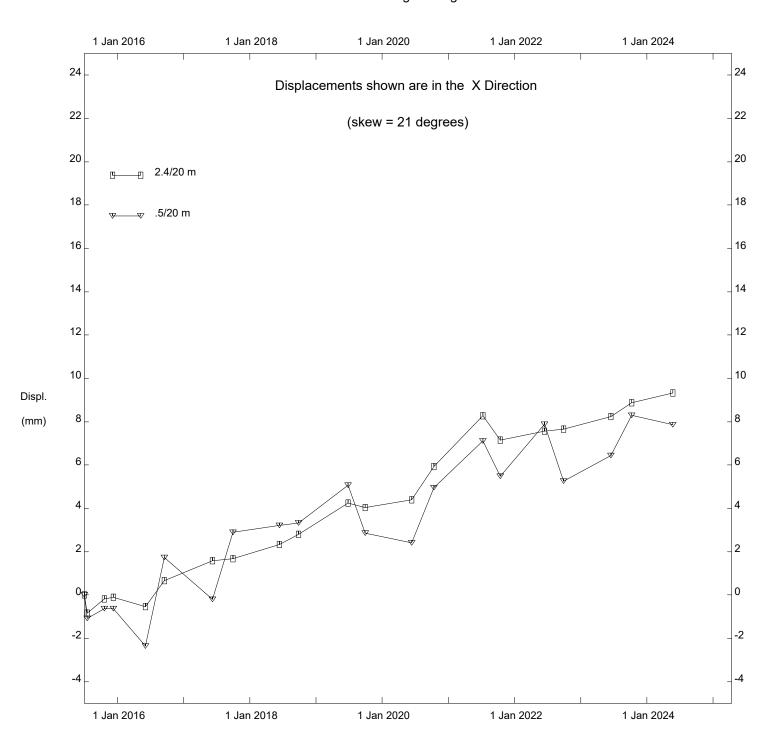
PH032 KM 58 (Post Construction), Inclinometer PK80

Alberta Transportation



PH032 KM 58 (Post Construction), Inclinometer PK80

Alberta Transportation



PH032 KM 58 (Post Construction), Inclinometer PK80

Thurber Engineering Ltd Deflection (mm) Deflection (mm) -50 0__ 25 50 __0 -25 0__ 12.5 25 __0 -25 -12.5**LEGEND** Initial 3 Jul 2015 WALER WALER 20 Jul 2015 2 21 Oct 2015 **PILE PILE** 10 Dec 2015 4 Jun 2016* 4 Clay (TILL), firm Clay (TILL), firm 16 Sep 2016 8 Jun 2017 6 6 6 29 Sep 2017 13 Jun 2018 26 Sep 2018 8 8 27 Jun 2019 Depth 30 Sep 2019 Depth (m) ₁₀ (m) 10 23 Jun 2020 10 13 Oct 2020 Clay, very hard Clay, very hard 10 Jul 2021 12 12 14 Oct 2021 12 14 Jun 2022 28 Sep 2022 14 14 14 16 Jun 2023 9 Oct 2023 23 May 2024 16 16 16 18 18 18 18 Silt Silt Ref. Elevation 491.508 m Sand Sand 20 20 20 20 -50 -25 25 50 -25 -12.5 12.5 25

PH032 Makeout (Post Construction), Inclinometer PM12

Alberta Transportation

Incremental Deflection

Direction A

Sets marked * include zero shift and/or rotation corrections.

Cumulative Deflection

Direction A

Thurber Engineering Ltd Deflection (mm) Deflection (mm) -50 0__ -25 25 50 __0 -25 0 -12.5 12.5 25 __0 **LEGEND** Initial 3 Jul 2015 WALER WALER 20 Jul 2015 2 2 21 Oct 2015 **PILE PILE** 10 Dec 2015 4 Jun 2016* 4 Clay (TILL), firm Clay (TILL), firm 16 Sep 2016 8 Jun 2017 6 6 6 29 Sep 2017 13 Jun 2018 26 Sep 2018 8 8 27 Jun 2019 Depth 30 Sep 2019 Depth (m) 10 23 Jun 2020 (m) 10 10 13 Oct 2020 Clay, very hard Clay, very hard 10 Jul 2021 12 12 12 14 Oct 2021 14 Jun 2022 28 Sep 2022 14 14 14 16 Jun 2023 9 Oct 2023 23 May 2024 16 16 16 18 18 ∐ 18 18 Silt Silt Ref. Elevation 491.508 m Sand Sand 20 20 20 20

PH032 Makeout (Post Construction), Inclinometer PM12

Alberta Transportation

-25

-12.5

Incremental Deflection

Direction B

12.5

25

Sets marked * include zero shift and/or rotation corrections.

50

25

-50

-25

Cumulative Deflection

Direction B

Thurber Engineering Ltd Deflection (mm) Deflection (mm) -50 0__ 25 50 __0 -25 0 12.5 25 __ 0 -25 -12.5**LEGEND** Initial 3 Jul 2015 WALER WALER 20 Jul 2015 2 2 21 Oct 2015 **PILE PILE** 10 Dec 2015 4 Jun 2016* 4 Clay (TILL), firm Clay (TILL), firm 16 Sep 2016 8 Jun 2017 6 6 6 29 Sep 2017 13 Jun 2018 26 Sep 2018 8 8 27 Jun 2019 Depth 30 Sep 2019 Depth (m) 10 23 Jun 2020 (m) 10 10 13 Oct 2020 Clay, very hard Clay, very hard 10 Jul 2021 12 12 14 Oct 2021 12 14 Jun 2022 28 Sep 2022 14 14 14 16 Jun 2023 9 Oct 2023 23 May 2024 16 16 16 ∐18 18 18 18 Silt Silt Ref. Elevation 491.508 m skew = 25deg Sand Sand 20 20 20 20 -50 -25 25 50 -25 -12.5 12.5 25

PH032 Makeout (Post Construction), Inclinometer PM12

Alberta Transportation

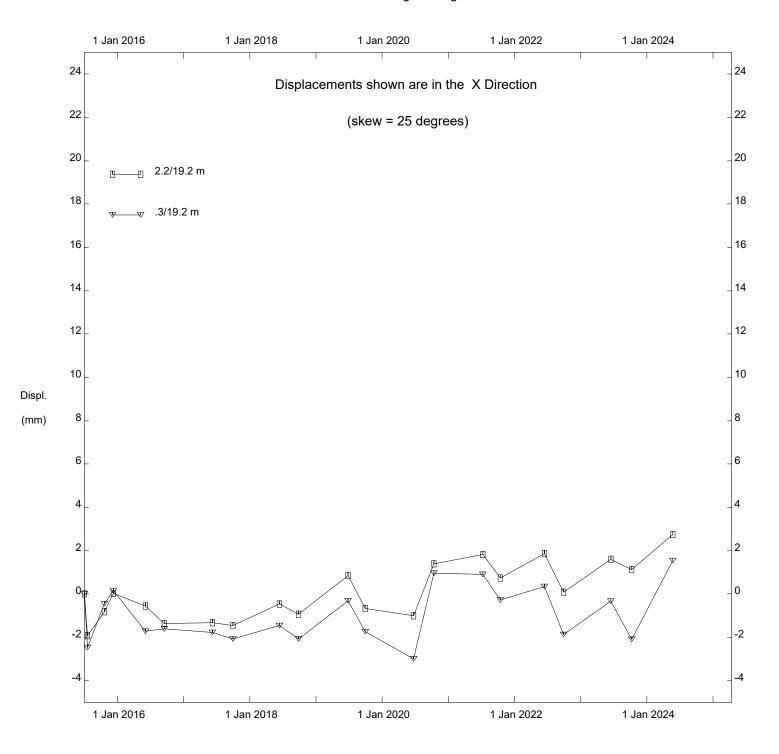
Incremental Deflection

Direction X

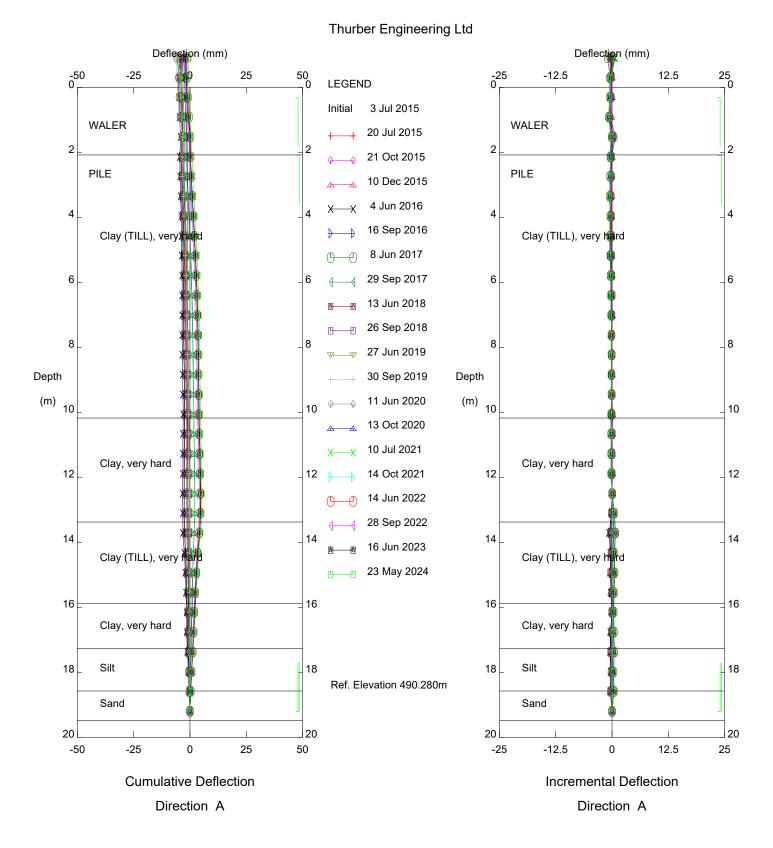
Sets marked * include zero shift and/or rotation corrections.

Cumulative Deflection

Direction X

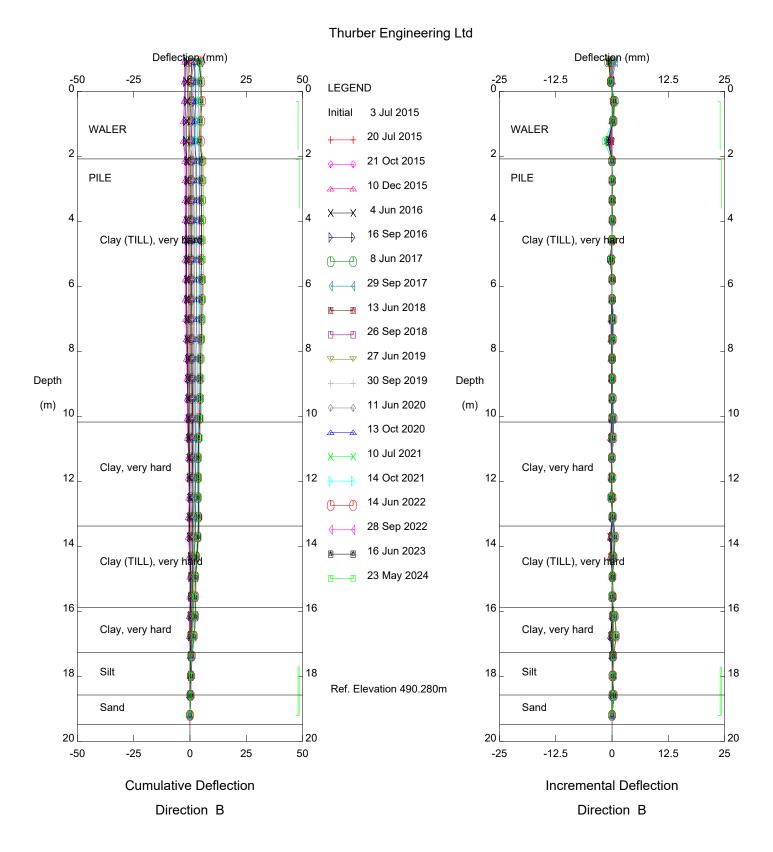


PH032 Makeout (Post Construction), Inclinometer PM12



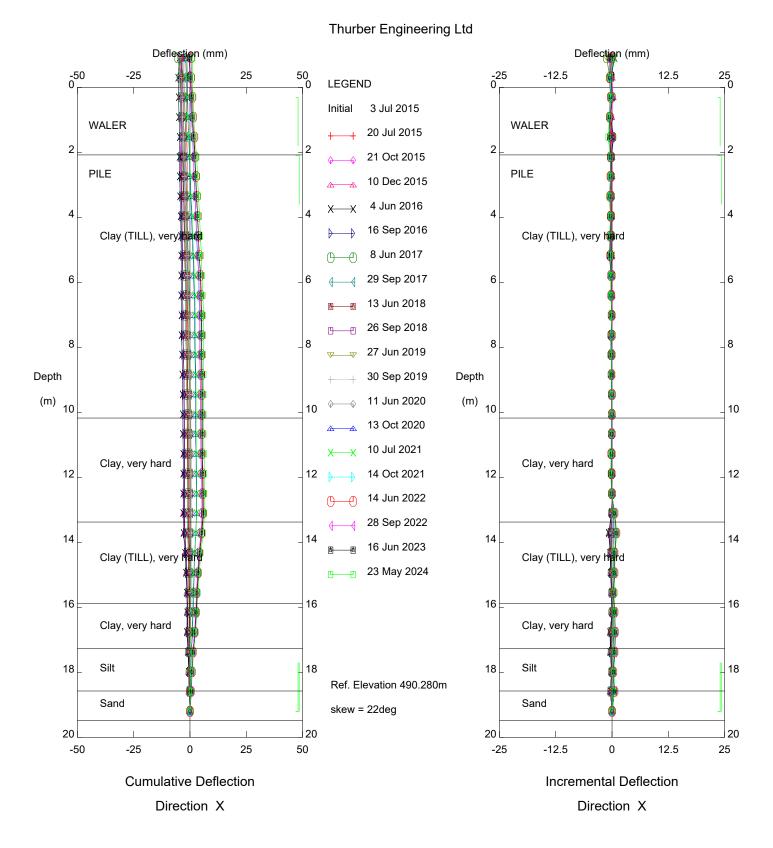
PH032 Makeout (Post Construction), Inclinometer PM24

Alberta Transportation



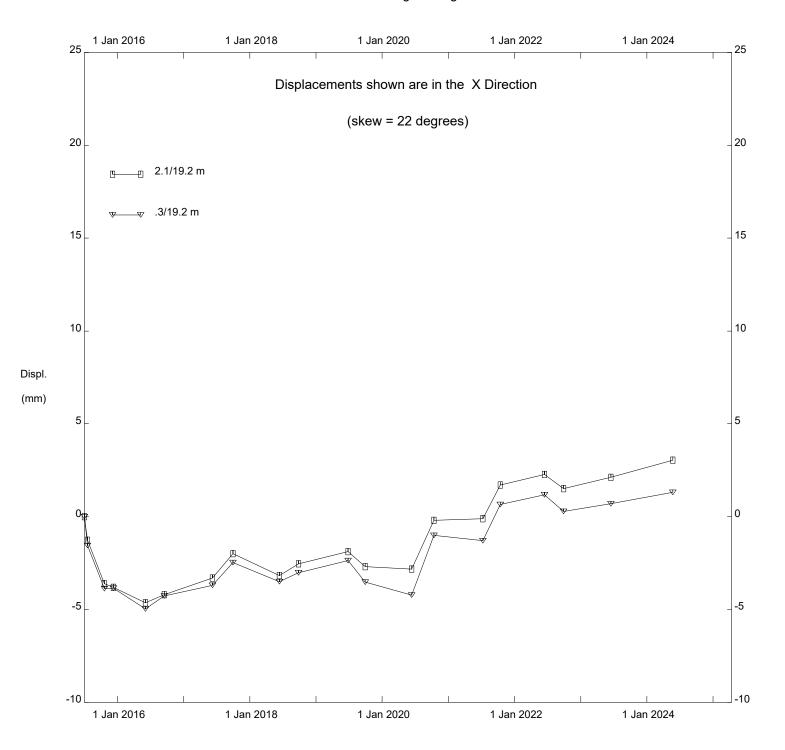
PH032 Makeout (Post Construction), Inclinometer PM24

Alberta Transportation



PH032 Makeout (Post Construction), Inclinometer PM24

Alberta Transportation



PH032 Makeout (Post Construction), Inclinometer PM24

FIGURE PH032-1
PIEZOMETERIC ELEVATIONS FOR HWY 744:04, JUDAH HILL MAKEOUT SLIDE

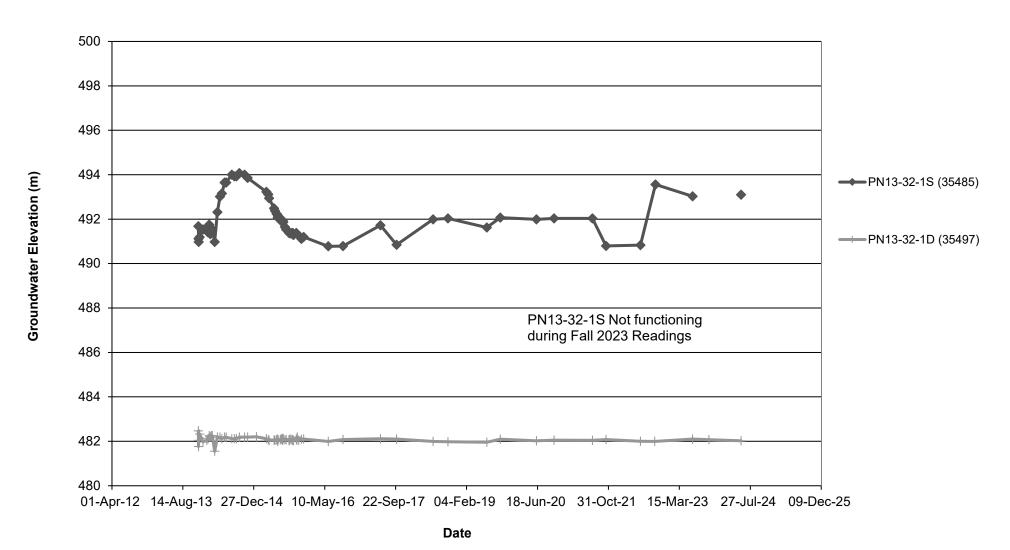


FIGURE PH032-2
PIEZOMETERIC DEPTHS FOR PH032-1: JUDAH HILL MAKEOUT SLIDE

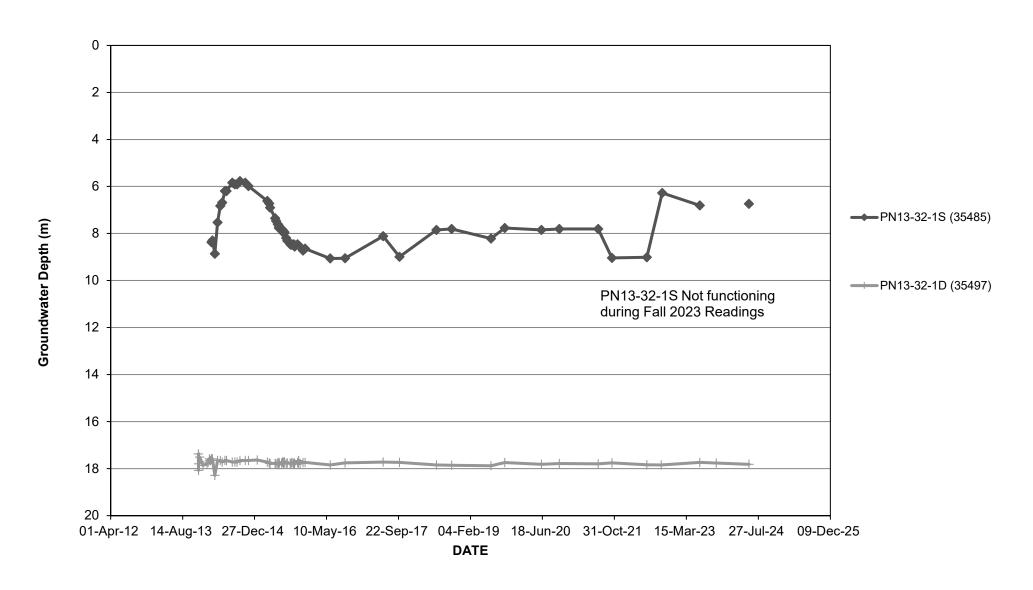


FIGURE PH032-3 LOAD CELL DATA FOR KM 58 PILE WALL

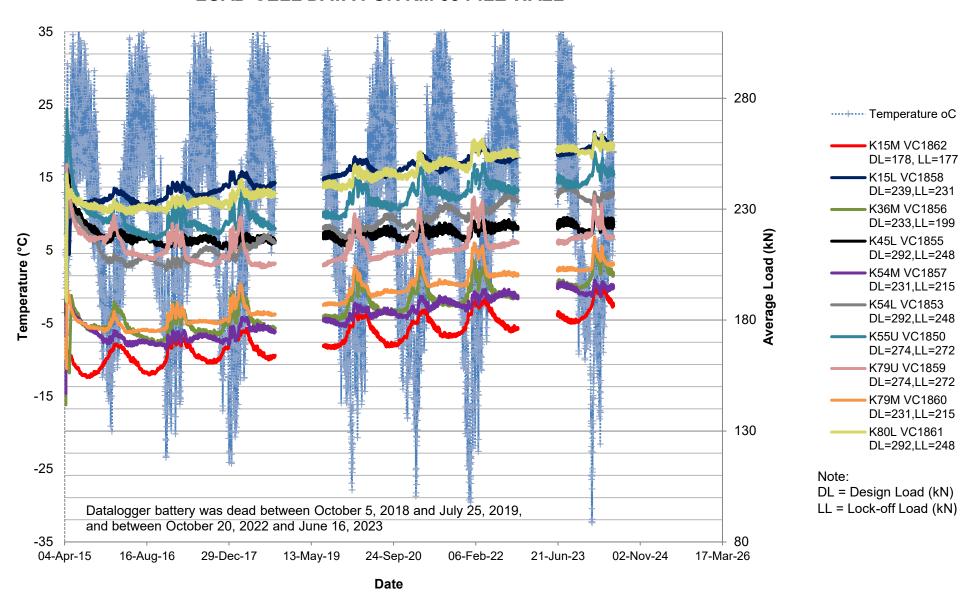


FIGURE PH032-4 LOAD CELL DATA FOR MAKEOUT PILE WALL

