ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS GRMP PEACE REGION – (PEACE RIVER DISTRICT) INSTRUMENTATION MONITORING - FALL 2024



Site Number	Location	Name	Hwy	km
PH032	HWY 744:04 C1 58.0	Makeout Slide - Judah Hill	744:04	Km 58.0
Legal Description	: 9-20-83-21 W5	UTM Co-ordinates		
		11U E 483237	N 622	29841

Current Monitoring:	Monitoring: 21-Sep-2024 Previous Monitoring		23-May-2024
Instruments Read By:	Mr. Niraj Regm	of Thurber	

Instruments Read During This Site Visit						
Slope Inclinometers (SIs): PK15, PK36, PK54 and PK80 (at KM 58 pile wall) PM12 and PM24 (at Makeout pile wall)	Pneumatic Piezometers (PN): PN13-32-1S and PN13 32 1D	Vibrating Wire Piezometers (VW):	Standpipe Piezometers (SP):			
Load Cell (LC): VC1850, VC1853, VC1855, VC1856, VC1857, VC1858, VC1859, VC1860, VC1861 and VC1862 (on selected anchors at KM 58 pile wall) VC1848, VC1849, VC1851, VC1852	Strain Gauges: N/A	SAAs:	Others:			
and VC1854 (on selected anchors at Makeout pile wall)						

Pneumatic Piezometers: RST C108 pneumatic piezometer readout	Vibrating Wire Piezometers:	Standpipe Piezometers:
Strain Gauges:	SAAs:	Others:
F P S	Viezometers: ST C108 pneumatic viezometer readout Strain Gauges:	Piezometers: Piezometers: RST C108 pneumatic Piezometers: viezometer readout Strain Gauges:

Discussion					
Zones of New	None				
Movement:					
Interpretation of Monitoring Results:	KM 58 Pile Wall Slope Indicators				
	PK15 showed a rate of movement of 0.3 mm/yr over the length of the pile and a rate of movement of 0.6 mm/yr over the combined length of the pile and waler since the spring of 2024 readings. Since the completion of construction, PK15 has shown a total cumulative deflection of 3.1 mm over the length of the pile in the downslope direction and a total cumulative movement of 3.6 mm in the downslope direction over the combined length of the pile and waler.				
	PK36 showed a rate of movement of 0.9 mm/yr over the length of the pile and a rate of movement of 3.0 mm/yr over the combined length of the pile and waler. Since the completion of construction, PK36 has shown total cumulative deflections of 5.6 mm in the downslope direction over the length of the pile				

and 6.1 mm in the downslope direction over the combined length of the pile and waler.
PK54 showed a rate of movement of 1.7 mm/yr over the length of the pile and a rate of movement of 3.9 mm/yr over the combined length of the pile and waler. Since the completion of construction, PK54 has shown total cumulative movements of 13.5 mm in the downslope direction over the length of the pile and 12.2 mm in the downslope direction over the combined length of the pile and waler.
PK80 showed a rate of movement of 3.1 mm/yr over the length of the pile and a rate of movement of 2.7 mm/yr over the combined length of the pile and waler. Since the completion of construction, PK80 has shown total cumulative movements of 10.4 mm of in the downslope direction over the length of the pile and 8.8 mm in the downslope direction over the combined length of the pile and waler.
The SIs at the KM 58 wall location show a current overall trend of slow downslope movement with average movement rates less than 2 mm/yr since completion of construction in 2015.
Makeout Slide Pile Wall Slope Indicators
PM12 showed no movement over the length of the pile and no discernible movement over the combined length of the pile and waler. Since the completion of construction, PM12 has shown total cumulative deflections of 2.2 mm in the downslope direction over the length of the pile and 0.4 mm in the upslope direction over the combined length of the pile and waler.
PM24 showed no movement over the length of the pile and a rate of movement of 1.2 mm/yr over the combined length of the pile and waler. Since the completion of construction, PM24 has shown total cumulative movements of 2.7 mm in the downslope direction over the length of the pile and 1.8 mm in the downslope direction over the combined length of the pile and waler.
After being pulled into the slope during the initial lock off of the anchors, the SIs at the Makeout wall location have show an overall trend of slow downslope movement with average movement rates less than 1 mm/yr since the end of construction in 2015. There have been minor seasonal changes in the wall displacement.
Piezometers
Pneumatic piezometers PN13-32-1S and PN13-32-1D showed increases in groundwater levels of 0.02 m and 0.01 m, respectively, since they were last read in the spring of 2024 readings. Pneumatic piezometer results are plotted in Figures PH032-1 (by elevation) and PH032-2 (by depth below ground surface) in Appendix A.
Load Cells
The load cells are connected to two dataloggers that are programmed to take two readings per day. Since the spring of 2024 readings, the load cells at the KM 58 wall showed minor changes in measured load ranging from a decrease of 5.94 kN in VC1862 (anchor K15M) to an increase of 4.16 kN in VC1859 (anchor K79U). Load cell VC1857 (K54M) and VC1853 (K54L) registered all time high measured loads between on July 18, 2024 and August 11, 2024, respectively. The anchors at the KM 58 wall show an overall trend of slowly increasing load, mainly with seasonally higher loads during the winter months. Load cells VC1862 (K15M) and VC1858 (K15L) show current loads that are 1.3 percent and 7.7 percent, respectively, above their SLS design loads.
At the Makeout wall, the load cells showed minor changes in measured load ranging from a decrease of 5.69 kN in VC1844 (anchor M12U) to an increase of 0.37 kN in VC1848 (anchor M12L). The load cells at the Makeout wall have also shown a trend of slowly increasing loads since the end of construction.

	with seasonably higher loads during the winter months. However, none of the							
	measured loads are over the SLS design loads.							
	The load cell average loads and temperatures are plotted for the KM 58 and Makeout walls on Figures PH032-3 and PH032-4, respectively, in Appendix A. The design and lock-off loads for each anchor are shown in the legends of the figures.							
	Overall, the SI and load cell data indicates that the pile walls have been effective at mitigating the landslide movements at this site and the measured deflections and anchor loads are within expected ranges. However, since the instruments at the KM 58 pile wall are showing a trend of downslope movement, combined with gradually increasing anchor loads, the instruments here should be monitored closely to ensure that the downslope movement can be further assessed well in advance of any required intervention.							
Future Work:	The instruments should be read again in the spring of 2025.							
Instrumentation Repairs:	None							
Additional	None							
Comments:								
	• Table PH032-1: Fall 2024 – HWY 744:04 Judah Hill (Makeout Slide)							
	Slope Inclinometer Instrumentation Reading Summary							
	Iable PH032-2: Fall 2024 – HWY 744:04 Judah Hill (Makeout Slide)							
	Toble DH022 2: Foll 2024 HW/V 744:04 Judeb Hill (Mekeout Slide)							
	Table PH032-3. Fall 2024 – HWY 744.04 Judan Hill (Makeoul Slide) Load Cell Instrumentation Reading Summary							
	Statement of Limitations and Conditions							
	Appendix A							
Attech menter	 Field Inspector's report 							
Attachments:	 Site Plan Showing Approximate Instrument Locations 							
	(Drawings No. 32121-PH032-1, 32121-PH032-2, and							
	32121-PH032-3)							
	 Pile Wall General Layout drawings 							
	 SI Reading Plots 							
	 Figure PH032-1 (Piezometric Elevations) 							
	 Figure PH032-2 (Piezometric Depths) Figure PH032-2 (Least Call Data for Key 50 Pile M(sll)) 							
	 Figure PH032-3 (Load Cell Data for Km 58 Pile Wall) Figure PH032 4 (Load Cell Data for Makagut Bile Wall) 							
	O FIGURE FILOSZ-4 (LUAU CEII DALA IOI MAKEOUL PILE WAII)							

We trust this report meets your requirements at present. If you have any questions, please contact the undersigned at your convenience.

Yours very truly, Thurber Engineering Ltd. Roger Skirrow, M.Sc., P. Eng. Senior Geotechnical Engineer

Lucas Green, P.Eng. Geotechnical Engineer



Table PH032-1: Fall 2024 – HWY 744:04 Judah Hill (Makeout Slide) Slope Inclinometer Instrumentation Reading Summary Date Monitored: September 21, 2024

INSTRUMENT #	DATE INITIALIZED (AFTER CONSTRUCTION)	TOTAL CUMULATIVE RESULTANT MOVEMENT AT NOTED DEPTH SINCE INITIAL READING (mm)	MAXIMUM RATE OF MOVEMENT (mm/yr)	CURRENT STATUS	DATE OF PREVIOUS READING	INCREMENTAL MOVEMENT SINCE PREVIOUS READING (mm)	RATE OF MOVEMENT (mm/yr)	CHANGE IN RATE OF MOVEMENT SINCE PREVIOUS READING (mm/yr)
			KM 58 V	VALL				
PK15 July 2, 2015	huhu 0, 0045	3.1 over 2.1 m to 13.7 m depth in 274° direction	17.3 in July 2015	Onerational	May 23,	0.1	0.3	0.3
	July 2, 2015	3.6 over 0.3 m to 13.7 m depth in 274° direction	29.1 in July 2015	Operational	2024	0.2	0.6	0.4
PK36 July 2, 2015	5.6 over 2.6 m to 16.6 m depth in 318° direction	3.4 in October 2020	Operational	May 23,	0.3	0.9	0.4	
	July 2, 2015	6.1 over 0.1 to 16.6 m depth in 318° direction	8.0 in September 2016	Operational	2024	1.0	3.0	1.1
PK54	luby 2, 2015	13.5 over 2.8 m to 20.4 m depth in 313° direction	12.0 in October 2020	Operational	May 23,	0.6	1.7	-0.1
	July 2, 2015	12.2 over 0.3 m to 20.4 m depth in 313° direction	13.3 in October 2020	Operational	2024	1.3	3.9	2.7

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.



Table PH032-1 – Continued... Fall 2024 – HWY 744:04 Judah Hill (Makeout Slide) Slope Inclinometer Instrumentation Reading Summary Date Monitored: September 21, 2024

INSTRUMENT #	DATE INITIALIZED	TOTAL CUMULATIVE RESULTANT MOVEMENT AT NOTED DEPTH SINCE INITIAL READING (mm)	MAXIMUM RATE OF MOVEMENT (mm/yr)	CURRENT STATUS	DATE OF PREVIOUS READING	INCREMENTAL MOVEMENT SINCE PREVIOUS READING (mm)	RATE OF MOVEMENT (mm/yr)	CHANGE IN RATE OF MOVEMENT SINCE PREVIOUS READING (mm/yr)
		10.4 over 2.4 m to 20.0 m depth in 262° direction	-20.2 in July 2015	Operational	May 23,	1.0	3.1	2.4
PK80 July 2,	July 2, 2013	8.8 over 0.5 m to 20.0 m depth in 262° direction	-26.4 in July 2015	Operational	2024	0.9	2.7	3.4
			MAKEOU	IT WALL				
DM12	July 2, 2015	2.2 over 2.2 m to 19.2 m depth in 316° direction	-41.3 in July 2015	Operational	, May 23,	No Discernible Movement	N/A	-4.3
PM12	July 3, 2015	0.4 over 0.3 m to 19.2 m depth in 316° direction	-52.8 in July 2015	Operational	2024	No Discernible Movement	N/A	-9.3
PM24	luk 2 2015	2.7 over 2.1 m to 19.2 m depth in 298° direction	-27.4 in July 2015	Operational	May 23,	No Discernible Movement	N/A	-1.9
	July 3, 2015	1.8 over 0.3 m to 19.2 m depth in 298° direction	-33.4 in July 2015	Operational	2024	0.4	1.2	0.6

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.



Table PH032-2: Fall 2024 – HWY 744:04 Judah Hill (Makeout Slide) Pneumatic Piezometer Instrumentation Reading Summary Date Monitored: September 21, 2024

INSTRUMENT #	DATE INITIALIZED	TIP DEPTH (m)	GROUND ELEV. (m)	CURRENT STATUS	HIGHEST MEASURED WATER LEVEL (m)	MEASURED PORE PRESSURE (kPa)	CURRENT GROUNDWATER ELEVATION (m)	PREVIOUS GROUNDWATER ELEVATION (m)	CHANGE IN WATER LEVEL SINCE PREVIOUS READING (m)
PN13-32-1S	November 30, 2013	9.14	499.84	Operational	493.56 in September 2022	23.6	493.12	493.10	0.02
PN13-32-1D	November 30, 2013	18.29	499.84	Operational	482.46 in December 2013	4.6	482.02	482.01	0.01

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.



Table PH032-3: Fall 2024 – HWY 744:04 Judah Hill (Makeout Slide) Load Cell Instrumentation Reading Summary Date Monitored: September 21, 2024

ANCHOR NUMBER	LOAD CELL SERIAL #	DESIGN LOAD / LOCK-OFF LOAD (kN)	MAXIMUM RECORDED LOAD (kN)	RECORDED LOAD ⁽¹⁾ (SEP. 21, 2024) (kN)	PREVIOUS RECORDED LOAD ⁽¹⁾ (MAY 23, 2024) (kN)	CHANGE IN LOAD SINCE PREVIOUS READING (KN)
			KM 58 WALL			
K15M	VC1862	178/177	194.23 on January 30, 2024	180.37	186.31	-5.94
K15L	VC1858	239/231	264.90 on January 28, 2024	257.34	257.87	-0.53
K36M	VC1856	233/199	214.91 on January 30, 2024	197.45	200.06	-2.61
K45L	VC1855	292/248	248.50 on April 20, 2015	223.60	222.31	1.29
K54M	VC1857	231/215	199.14 on July 18, 2024	195.78	194.49	1.29
K54L	VC1853	292/248	243.56 on August 11, 2024	240.78	236.67	4.11
K55U	VC1850	274/272	275.28 on April 17, 2015	243.02	245.24	-2.22
K79U	VC1859	274/272	250.27 on April 16, 2015	221.22	217.06	4.16
K79M	VC1860	231/215	217.55 on January 30, 2024	206.39	205.10	1.29
K80L	VC1861	292/248	264.09 on January 28, 2024	259.06	257.42	1.64

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.

(1) Load cell data is recorded twice daily with datalogger on site. Dataloggers' data are uploaded twice annually during instrumentation readings. See Figure PH032-3 for combined historical instrument readings.



 Table PH032-3 – Continued...Fall 2024 – HWY 744:04 Judah Hill (Makeout Slide) Load Cells Instrumentation Reading Summary

 Date Monitored: September 21, 2024

ANCHOR NUMBER	LOAD CELL SERIAL #	DESIGN LOAD / LOCK-OFF LOAD (kN)	MAXIMUM RECORDED LOAD (kN)	RECORDED LOAD ⁽¹⁾ (SEP. 21, 2024) (kN)	PREVIOUS RECORDED LOAD ⁽¹⁾ (MAY 23, 2024) (kN)	CHANGE IN LOAD SINCE PREVIOUS READING (kN)			
MAKEOUT WALL									
M12U	VC1854	274/272	277.02 on March 18, 2022	248.22	253.91	-5.69			
M12M	VC1849	231/215	213.90 on March 25, 2015	198.61	200.74	-2.13			
M12L	VC1848	292/248	253.28 on March 22, 2023	244.52	244.15	0.37			
M24U	VC1851	274/272	271.81 on March 25, 2015	245.03	248.71	-3.68			
M24M	VC1852	231/215	217.10 on March 25, 2015	182.57	185.60	-3.03			

Drawings 32121-PH032-1~3 in Appendix A provide a sketch of the approximate location of the monitoring instrumentation for this site.

(1) Load cell data is recorded twice daily with datalogger on site. Dataloggers data are uploaded twice annually during instrumentation readings. See Figure PH032-4 for combined historical instrument readings.

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STATEMENT OF LIMITATIONS AND CONDITIONS

1. STANDARD OF CARE

This Report has been prepared in accordance with generally accepted engineering or environmental consulting practices in the applicable jurisdiction. No other warranty, expressed or implied, is intended or made.

2. COMPLETE REPORT

All documents, records, data and files, whether electronic or otherwise, generated as part of this assignment are a part of the Report, which is of a summary nature and is not intended to stand alone without reference to the instructions given to Thurber by the Client, communications between Thurber and the Client, and any other reports, proposals or documents prepared by Thurber for the Client relative to the specific site described herein, all of which together constitute the Report.

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3. BASIS OF REPORT

The Report has been prepared for the specific site, development, design objectives and purposes that were described to Thurber by the Client. The applicability and reliability of any of the findings, recommendations, suggestions, or opinions expressed in the Report, subject to the limitations provided herein, are only valid to the extent that the Report expressly addresses proposed development, design objectives and purposes, and then only to the extent that there has been no material alteration to or variation from any of the said descriptions provided to Thurber, unless Thurber is specifically requested by the Client to review and revise the Report in light of such alteration or variation.

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- a) Nature and Exactness of Soil and Contaminant Description: Classification and identification of soils, rocks, geological units, contaminant materials and quantities have been based on investigations performed in accordance with the standards set out in Paragraph 1. Classification and identification of these factors are judgmental in nature. Comprehensive sampling and testing programs implemented with the appropriate equipment by experienced personnel may fail to locate some conditions. All investigations utilizing the standards of Paragraph 1 will involve an inherent risk that some conditions will not be detected and all documents or records summarizing such investigations will be based on assumptions of what exists between the actual points sampled. Actual conditions may vary significantly between the points investigated and the Client and all other persons making use of such documents or records with our express written consent should be aware of this risk and the Report is delivered subject to the express condition that such risk is accepted by the Client and such other persons. Some conditions are subject to change over time and those making use of the Report should be aware of this possibility and understand that the Report only presents the conditions at the sampled points at the time of sampling. If special concerns exist, or the Client has special considerations or requirements, the Client should disclose them so that additional or special investigations may be undertaken which would not otherwise be within the scope of investigations made for the purposes of the Report.
- b) Reliance on Provided Information: The evaluation and conclusions contained in the Report have been prepared on the basis of conditions in evidence at the time of site inspections and on the basis of information provided to Thurber. Thurber has relied in good faith upon representations, information and instructions provided by the Client and others concerning the site. Accordingly, Thurber does not accept responsibility for any deficiency, misstatement or inaccuracy contained in the Report as a result of misstatements, omissions, misrepresentations, or fraudulent acts of the Client or other persons providing information relied on by Thurber. Thurber is entitled to rely on such representations, information and instructions and is not required to carry out investigations to determine the truth or accuracy of such representations, information and instructions.
- c) Design Services: The Report may form part of design and construction documents for information purposes even though it may have been issued prior to final design being completed. Thurber should be retained to review final design, project plans and related documents prior to construction to confirm that they are consistent with the intent of the Report. Any differences that may exist between the Report's recommendations and the final design detailed in the contract documents should be reported to Thurber immediately so that Thurber can address potential conflicts.
- d) Construction Services: During construction Thurber should be retained to provide field reviews. Field reviews consist of performing sufficient and timely observations of encountered conditions in order to confirm and document that the site conditions do not materially differ from those interpreted conditions considered in the preparation of the report. Adequate field reviews are necessary for Thurber to provide letters of assurance, in accordance with the requirements of many regulatory authorities.

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ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS GRMP (CON0022164) PEACE REGION (PEACE RIVER DISTRICT) INSTRUMENTATION MONITORING RESULTS

FALL 2024

APPENDIX A DATA PRESENTATION

SITE PH032: HWY 744:04, JUDAH HILL (MAKEOUT SLIDE)

ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS PEACE REGION (PEACE RIVER DISTRICT) INSTRUMENTATION MONITORING FIELD SUMMARY (PH032) FALL 2024

Location: Makeout Slide - Judah Hill (HWY 744:04	C1 57.924) Readout: RST PN C108 Unit 4
File Number: 32121	Casing: 2.75
Probe: RST SET 5R	Temp: 10
Cable: RST SET 5R	Read by: NRM/NKR

SLOPE INCLINOMETER (SI) READINGS

SI#	GPS	Location	Date	Stickup	Depth from top	Magn. North	Cu	Current Bottom		Probe/	Size	Remarks	
	(UT	CM 11)		(m)	of Casing (ft)	A+ Groove	Depth Readings		Reel	(")			
	Easting (m)	Northing (m)					A+	A-	B+	B-	#		
PK15	483237	6229841	21-Sep-24	1.21	48 to 2	245	383	-372	540	-550	5R/5R	2.75	
PK36	483225	6229863	21-Sep-24	0.8	56 to 2	310	-202	213	-38	25	5R/5R	2.75	
PK54	483214	6229882	21-Sep-24	1.2	70 to 2	300	706	-697	-159	143	5R/5R	2.75	
PK80	483199	6229909	21-Sep-24	0.99	68 to 2	225	-410	421	219	-236	5R/5R	2.75	
PM12	483157	6229989	21-Sep-24	1.18	66 to 2	275	-850	856	839	-845	5R/5R	2.75	
PM24	483151	6230002	21-Sep-24	1.22	66 to 2	260	499	-487	513	-513	5R/5R	2.75	

PN#	GPS Locatio	on (NAD83)	Date	Reading	Identification
	Easting (m) Northing (m)			(kPa)	Number
PN13-32-1S	483205	6229901	21-Sep-24	23.8	35485
PN13-32-1D	483205	6229901	21-Sep-24	4.5	35497

VC #	GPS Location (UTM 11)		Datalogger	Date		
	Easting (m)	Northing (m)	Serial #		Comment	
VC1850					Downloaded	
VC1853					Downloaded	
VC1855					Downloaded	
VC1856					Downloaded	
VC1857			DST 2034	Doy Doy		
VC1858			K31 2034		Downloaded	
VC1859					Downloaded	
VC1860				21-Sep-24	Downloaded	
VC1861					Downloaded	
VC1862					Downloaded	
VC1848					Downloaded	
VC1849					Downloaded	
VC1851			RST 2036		Downloaded	
VC1852]		Downloaded	
VC1854					Downloaded	

VIBRATING WIRE LOAD CELL (VC) READINGS

P

INSPECTOR REPORT					
N 13-32-1S Reading takes a long time to stabilize					







GENERAL NOTES

- ALL DIMENSIONS SHOWN ON THE PILE WALL GENERAL LAYOUT ARE GIVEN IN METRES. ALL OTHER PILE WALL DRAWINGS ARE DIMENSIONED IN MILLIMETRES EXCEPT FOR STATIONS AND ELEVATIONS WHICH ARE GIVEN IN METRES.
- DRAWING SCALES ARE BASED ON PLOTTING FULL SIZE (22"x34")

DESIGN

• CAN/CSA S6-06 CANADIAN HIGHWAY BRIDGE DESIGN CODE + SUPPLEMENTS S6S1, S6S2, AND S6S3

475 ED AWINGS IONS.									
FIELD REVIEW ENGINEER DRIGINAL DOCUMENT STAMPED AND SIGNED BY: S. L. BROWN ON DECISION		KM PILE I GEN	58 LANDS RETAINING NERAL LA`	GL I DE G WALL YOUT			A	(bert i Transporta	tion
ON: DEC 16, 2016	REGION	SITE No.	PLAN No.	PROJECT	CONTRACT No.	SHEET	2.5	0 HORIZONTAL	 5





GENERAL NOTES

- ALL DIMENSIONS SHOWN ON THE PILE WALL GENERAL LAYOUT ARE GIVEN IN METRES. ALL OTHER PILE WALL DRAWINGS ARE DIMENSIONED IN MILLIMETRES EXCEPT FOR STATIONS AND ELEVATIONS WHICH ARE GIVEN IN METRES.
- DRAWING SCALES ARE BASED ON PLOTTING FULL SIZE (22"x34")

DESIGN

• CAN/CSA S6-06 CANADIAN HIGHWAY BRIDGE DESIGN CODE + SUPPLEMENTS S6S1, S6S2, AND S6S3

s	QUANTITY ESTIMATE	
VIEW ENGINEER L DOCUMEN' IPED AND NED BY: PROWN	. MAKEOUT LANDSLIDE PILE RETAINING WALL GENERAL LAYOUT	Alberta Transportation

RD-19078-C

DRILL RIG SET-UP

PILE INSTALLATION

kg

m³

m³

PILE

m

UNIT

PLAN No. PROJECT CONTRACT No. SHEET

15153

45 of 48

744:04

87 570

100

730

35

644

-

-

-

-

-

TOT EST AS CONST

REINFORCING STEEL PLAIN

ITEM

SITE No.

PH72

CONCRETE - CLASS C

REGION

PEACE

DRILLED

PILES

CONCRETE

CONCRETE - CLASS PILE

Thurber Engineering Ltd. Deflection (mm) Deflection (mm) -50 0___ 25 50 ___0 -25 -12.5 12.5 25 __0 -25 LEGEND Initial 2 Jul 2015 1 1 18 Jul 2015 1 WALER WALER 21 Oct 2015 2 2 2 2 10 Dec 2015 Clay FILL PILE Clay FILL PILE 3 Jun 2016 X X 3 3 3 3 16 Sep 2016 Clay Soft to Firm Clay Soft to Firm 8 Jun 2017 Д 4 4 4 4 29 Sep 2017 13 Jun 2018 5 5 5 5 Sand Sand 26 Sep 2018 Щ 27 Jun 2019 6 6 6 6 30 Sep 2019 Depth Depth 11 Jun 2020 (m) 7 (m) 7 7 7 13 Oct 2020 10 Jul 2021 8 8 8 8 Clay (Till) Very Stif Clay (Till) Very Stiff 13 Oct 2021 14 Jun 2022 Д 9 9 ብ 9 9 28 Sep 2022 16 Jun 2023 10 10 10 10 9 Oct 2023 Γ4-11 23 May 2024 11 11 11 21 Sep 2024 Clay Very Stiff Clay Very Stiff 12 12 12 12 Ref. Elevation 504.201m 13 13 13 13 Clay Hard Clay Hard 14 14 14 14 -50 -25 0 25 50 -25 -12.5 0 12.5 25 **Cumulative Deflection** Incremental Deflection Direction A Direction A







Thurber Engineering Ltd. Deflection (mm) Deflection (mm) -50 0___ 25 50 ___0 -25 -12.5 12.5 25 __0 -25 LEGEND Initial 2 Jul 2015 1 1 18 Jul 2015 1 WALER WALER 21 Oct 2015 2 2 2 2 10 Dec 2015 Clay FILL PILE Clay FILL PILE 3 Jun 2016 X × 3 3 3 3 16 Sep 2016 Clay Soft to Firm Clay Soft to Firm 8 Jun 2017 Д 4 4 4 4 29 Sep 2017 13 Jun 2018 5 5 5 5 Sand Sand 26 Sep 2018 Щ 27 Jun 2019 6 6 6 6 30 Sep 2019 Depth Depth 11 Jun 2020 (m) 7 (m) 7 7 7 13 Oct 2020 10 Jul 2021 8 8 8 8 Clay (Till) Very Stiff Clay (Till) Very Stiff 13 Oct 2021 14 Jun 2022 Д 9 9 ብ 9 9 28 Sep 2022 16 Jun 2023 10 10 10 10 9 Oct 2023 Γ. 11 23 May 2024 11 11 11 21 Sep 2024 Clay Very Stiff Clay Very Stiff 12 12 12 12 Ref. Elevation 504.201m 13 13 13 13 Clay Hard Clay Hard skew = 13deg 14 14 14 14 -50 -25 0 25 50 -25 -12.5 0 12.5 25 **Cumulative Deflection** Incremental Deflection Direction X Direction X





PH032 KM 58 (Post Construction), Inclinometer PK15







PH032 KM 58 (Post Construction), Inclinometer PK36







PH032 KM 58 (Post Construction), Inclinometer PK36















PH032 KM 58 (Post Construction), Inclinometer PK54















PH032 KM 58 (Post Construction), Inclinometer PK80



PH032 Makeout (Post Construction), Inclinometer PM12

Alberta Transportation

Sets marked * include zero shift and/or rotation corrections.

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PH032 Makeout (Post Construction), Inclinometer PM12

Alberta Transportation

Sets marked * include zero shift and/or rotation corrections.

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PH032 Makeout (Post Construction), Inclinometer PM12

Alberta Transportation

Sets marked * include zero shift and/or rotation corrections.

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PH032 Makeout (Post Construction), Inclinometer PM12



PH032 Makeout (Post Construction), Inclinometer PM24



PH032 Makeout (Post Construction), Inclinometer PM24







PH032 Makeout (Post Construction), Inclinometer PM24





Date





Groundwater Depth (m)

FIGURE PH032-3 LOAD CELL DATA FOR KM 58 PILE WALL





FIGURE PH032-4 LOAD CELL DATA FOR MAKEOUT PILE WALL

Date