ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS GEOHAZARD ASSESSMENT PROGRAM PEACE REGION – SWAN HILLS 2024 INSPECTION



Site Number	Location	Name	Hwy	km		
SH023-10	Little Smoky River	Little Smoky River Valley, North Hill – Site #10	744:02	20.20-20.40		
Legal Description		UTM Co-ordinates	UTM Co-ordinates			
NE21-76-22-W5M		11U E 478,074	N 6	,161,918		

	Date	PF	CF	Total
Previous Inspection:	1-Jun-2022	10	3	30
Current Inspection:	4-Jun-2024	10	3	30
Road AADT:	270		Year:	2023
Increased Dry	Rishi Adhikari, TEC		Ken Froese, Thurber	
Inspected By:	Robert Senior, TEC		Roger Skirrow, Thurber	
Devent Attechmenter	Photographs			
Report Attachments:	Plans		Maintenance Items	

Primary Site Issue:	Highway traverses deep-seated, retrogressive ongoing creep movements due partly to erosion at Smoky River and Peavine Creek resulting in crackir the pavement surface at numerous locations. App highway crosses this unstable north valley slope. above and 480 m away from the Peavine Creek.	t toe by the Little ng and sagging of prox. 4 km of the	
Dimensions:	135 m length of highway affected by cracking and distortion		
Date of Remediation:	2000: Subdrain pipe from Site #11 installed in downslope ditch. 2006: Both ditches regraded and lined with riprap.		
Maintenance:	Routine ACP crack sealing, milling, and patching, when required. 2013: ACP patch placed. 2019: Patch over south portion 2020: Line painting 2021: Highway overlay (50 mm)		
Observations:	Description	Worsened?	
☑ Pavement Distress	Longitudinal and traverse cracks have reflected through the ACP and have extended. New faint cracks appearing.		
☑ Slope Movement	Site is located on an active deep-seated landslide moving toward the Peavine Creek. This highway crosses over and along a main scarp at this geohazard site.		
⊠ Erosion	A gully below the riprap lined ditch outlet at the south end of site extends southward into a sag pond located about 110 m from the highway. Erosion gullies have formed at the culvert outlet and at end of riprap. Continued erosion at inlet of culvert.		
⊠ Seepage	Upslope ditch at northeast end of site has been historically wet and poorly drained.		
⊠ Bridge/Culvert	SWSP culverts installed at km 20.22 and at km 20.38. Outlet of culvert at km 20.38 is 50 percent full and ponded water was observed at the outlet.		
□ Other			

Instrumentation: None.

Assessment:

The overall valley slope is moving as several separate slide blocks in response to the toe erosion and downcutting of two different rivers resulting in numerous scarps, sag ponds, and differential movement zones going in slightly different directions. The highway intersects the scarps of these blocks at several locations resulting in an uneven highway surface and pavement cracking.

Although this Site #10 is located on the unstable valley slope with a significant sag pond located below the site, landslide movements have not been consistently observable at this site with most cracking being transverse or longitudinal. There is a noticeable dip in the highway profile which could also be related to soft subgrade soils given the historically wet upslope ditch. Previously observed vertical distortions and angled cracks were not present during the 2022 inspection since the highway was overlaid in 2021. Continued reflection of the previous crack pattern was observed during the 2024 inspection.

Historically, there has also been problems with erosion of the ditches following high precipitation events. The erosion gully forming away from the highway below the riprap apron had noticeably deteriorated since 2018. SWSP culverts were installed in 2021 at km 20.22 and at km 20.38. Culvert at km 20.38 is 50 percent full and ponded water was observed at the outlet. Standing water was observed in the east ditch which is an indication of poor drainage at the site.

Recommendations:

Short-Term:

 Road maintenance should continue as necessary to maintain a safe roadway surface and may consist of ACP milling, patching, and crack sealing.

Medium-Term:

 Minor regrading could be carried out to allow drainage of the ponded water at the north culvert and standing water observed in the east ditch.

Long-Term:

It is understood that, at this time, the only long-term remediation option under consideration is realignment of the entire north hill section of Highway 744 and this study is currently being undertaken by CIMA+. Consideration is also being given to a shorter realignment which would include both of the SH023 sites as they currently require frequent maintenance.

Ongoing Investigation:

• It is recommended that the biennial geohazard inspection should continue as scheduled.

Closure:

It is a condition of this letter report that Thurber's performance of its professional services will be subject to the attached Statement of Limitations and Conditions.

Roger Skirrow, P.Eng. Senior Geotechnical Engineer

Mark Gallego, P.Eng. Geotechnical Engineer



STATEMENT OF LIMITATIONS AND CONDITIONS

1. STANDARD OF CARE

This Report has been prepared in accordance with generally accepted engineering or environmental consulting practices in the applicable jurisdiction. No other warranty, expressed or implied, is intended or made.

2. COMPLETE REPORT

All documents, records, data and files, whether electronic or otherwise, generated as part of this assignment are a part of the Report, which is of a summary nature and is not intended to stand alone without reference to the instructions given to Thurber by the Client, communications between Thurber and the Client, and any other reports, proposals or documents prepared by Thurber for the Client relative to the specific site described herein, all of which together constitute the Report.

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The Report has been prepared for the specific site, development, design objectives and purposes that were described to Thurber by the Client. The applicability and reliability of any of the findings, recommendations, suggestions, or opinions expressed in the Report, subject to the limitations provided herein, are only valid to the extent that the Report expressly addresses proposed development, design objectives and purposes, and then only to the extent that there has been no material alteration to or variation from any of the said descriptions provided to Thurber, unless Thurber is specifically requested by the Client to review and revise the Report in light of such alteration or variation.

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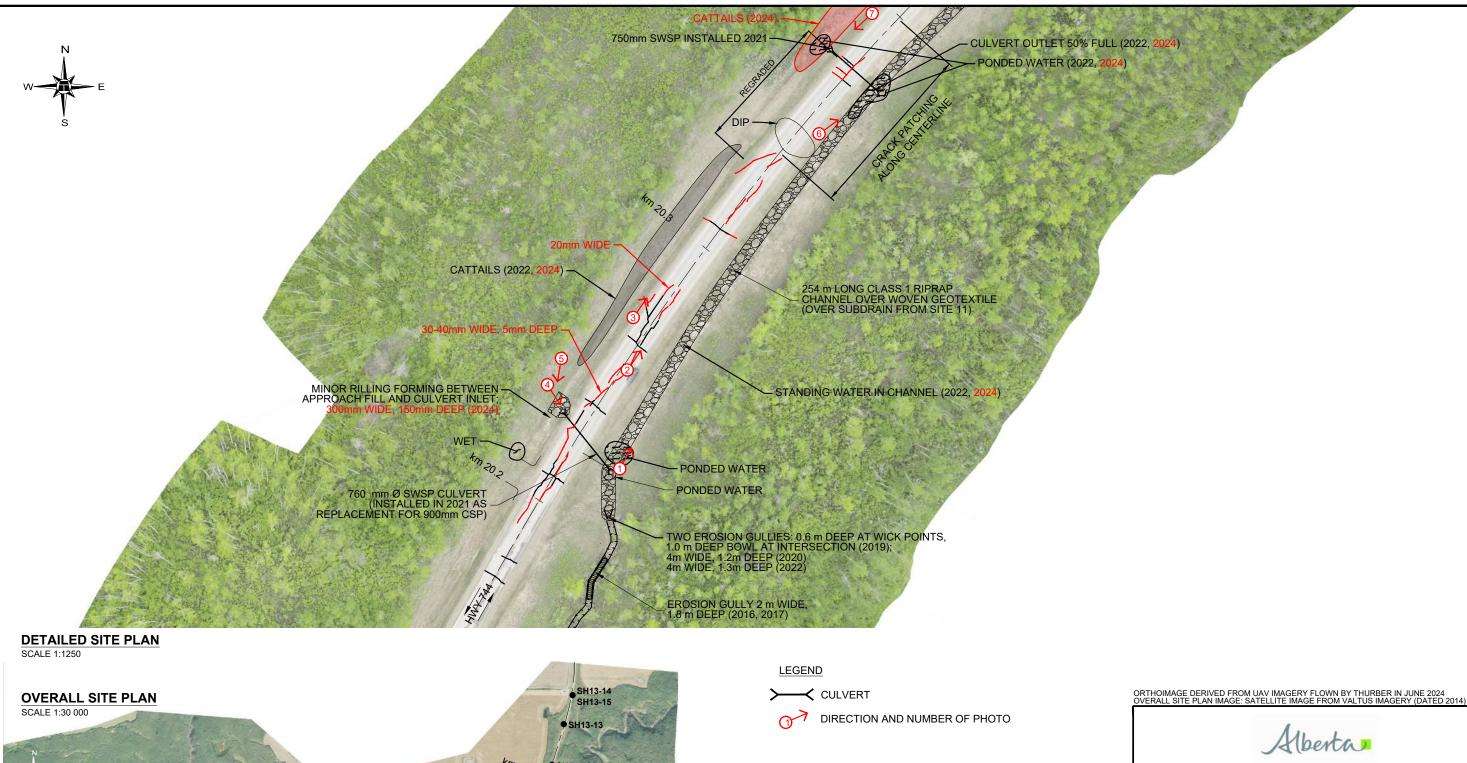
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- c) Design Services: The Report may form part of design and construction documents for information purposes even though it may have been issued prior to final design being completed. Thurber should be retained to review final design, project plans and related documents prior to construction to confirm that they are consistent with the intent of the Report. Any differences that may exist between the Report's recommendations and the final design detailed in the contract documents should be reported to Thurber immediately so that Thurber can address potential conflicts.
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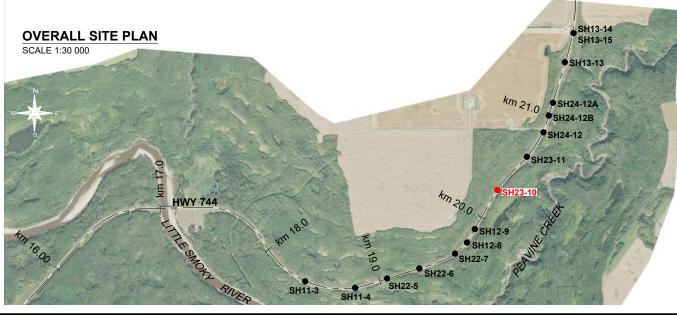
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NOTES

- 1. FEATURE LOCATIONS ARE APPROXIMATE.
- 2. JUNE 2024 OBSERVATIONS SHOWN IN RED.
- 3. CRACK AND PATCH PATTERNS RESET AS HWY 744 WAS OVERLAID IN SUMMER 2021.
- 4. CULVERT LOCATIONS TAKEN FROM MCINTOSH PERRY AS-BUILT DRONE SURVEY (JULY 2021).

20 30 10 50 60 70 m SCALE 1:1250

PEACE REGION (SWAN HILLS)

SH023-10: HWY 744:02 LITTLE SMOKY RIVER VALLEY 2024 SITE INSPECTION PLAN

DRAWN BY	KLP
DESIGNED BY	MG
APPROVED B	Y RKS
SCALE	AS SHOWN
DATE	SEPTEMBER 2024
FILE No.	32121



DWG No. 32121-SH023-10





Photo 1 – Looking northeast along downslope ditch.



Photo 2 – Looking northeast at cracking that is continuing to surface around km 20.26 (just northeast of the culvert).





Photo 3 – Looking northeast at potential scarp crack northeast of Photo 3.



Photo 4 – Looking at inlet of culvert at km 20.22. Note minor rill on the right-hand side of the photo.





Photo 5: Erosion adjacent to km 20.22 culvert inlet (right side of Photo 5).



Photo 6: Ponding water in regraded downslope ditch at the km 20.38 culvert outlet.





Photo 7: Poor drainage and ponding water at the km 20.38 culvert inlet.