ALBERTA TRANSPORTATION
GEOHAZARD ASSESSMENT PROGRAM
PEACE REGION – PEACE RIVER/HIGH LEVEL
2016 INSPECTION

THURBER ENGINEERING LTD.

Site Number	Location	Name	Hwy	km
PH 77-1 and 77-2	West of Fairview	Sites E of Hines Creek Bridge	682:02	12.5-12.8
Legal Description		**UTM Co-ordinates (NAD 83)**		
NE35-81-5-W6		11V N 6215000 E 396650		

	Date	PF	CF	Total
Previous Inspection:	June 8, 2015	11 13	4 4	44 (Site 1) 52 (Site 2)
Current Inspection:	June 1, 2016	11 13	4 4	44 (Site 1) 52 (Site 2)
Road AADT:	200		**Year:**	2015
Inspected By:	Don Proudfoot, Barry Meays (Thurber); Ed Szmata, Ken Szmata, Rocky Wang (AT)			
Report Attachments:	☑ Photographs ☑ Plans ☐ Maintenance Items			

Primary Site Issue:	SITE 1: Creek bank slumping caused by creek erosion continues to retrogress and is now causing cracks to appear in the highway about 150 m east of the bridge. A shallow sideslope slide also exists at the east end. SITE 2: Settlement in the highway and an active landslide through the south embankment are causing distress to the BF culvert and pavement. Channel and ditch erosion has created a slide around the culvert inlet.		
Dimensions:	SITE 1: Slide dimensions ~100m wide along the embankment parallel to the highway x ~30m long. SITE 2: South embankment slide ~40m wide x 75m long extending to river. North embankment slope slide ~30m x ~30m. Erosion ~30m long in the ditch TRM's, plus ~25 m of upstream channel erosion.		
Date of any remediation:	Site 2 was repaired in the fall of 2014 with a new 1.65m dia. SWSP (WSP = Consultant, In-Line = Contractor).		
Maintenance:	Crack sealing.	**Worsened?**	
Observations:	**Description**	**Yes**	**No**
☑ Pavement Distress	At Site 1, a 30m long (30 to 80mm wide) crack and a 15m long crack (near the east end) exist in the eastbound driving lane. A dip exists outside the longer crack extending across the hwy. At Site 2, cracks and a noticeable dip exist across the highway overtop the SWSP culvert.	☐	☐
☑ Slope Movement	At Site 1: The Creek has eroded the toe of the south embankment, and a slide scarp has developed parallel to the highway, and is as close as 8m from the pavement. At the east end, the 10m wide shallow sideslope slide has moved downslope, and created a 0.6m high scarp (at 3.2m from the guardrail) and a 0.6m high toe push. At Site 2: A landslide is	☑	☐

	developing through the south embankment and has reportedly deformed the culvert. There is a 0.3m high scarp located 5.1m from the white shoulder line. North of the highway, a 0.6m wide scarp crack with a 0.2m drop has developed above the erosion near the culvert inlet.		
☑ Erosion	At Site 1, erosion is occurring at the toe of the slope along the creek's edge. At Site 2, the TRM south of the highway along the east runoff ditch contains areas of erosion and undermining. North of the highway serious erosion exists in the channel leading up to the culvert inlet, which has undermined the downstream end of the gabions, and there is also 0.6m deeper erosion at the top of the gabions to TRM transition on the east ditch. The channel upstream of the culvert inlet is also downcut up to 3m deep x 5m wide.	☐	☐
☐ Seepage		☐	☐
☑ Bridge/Culvert Distress	The ends of the 1.65 m diameter SWSP (BF75380) at Site 2 did not appear to be visibly damaged at the time of the inspection, however AT noted that distortions within the culvert were noted during the latest bridge inspection.	☐	☐
☐ Other		☐	☐

Instrumentation:
None

Background/Assessment (Refer to Figures PH77-1, -2, -1/2)**:**
The existing bridge file management system records indicate that BF75380 **at Site 2** consisted of a SPE having an in service date of 1961 (the size of the SPE was not identified), with a 15.8 m deck height. This site was repaired in 2012 with a new 1.65 m dia. SWSP culvert, and there was some mention of a slide at the site at that time.

The cracks and subsidence in the south embankment slope are likely evidence that a landslide has formed in the slope at this location. This is supported by reports from AT that the culvert barrel has become distorted, which suggest that the landslide is deep seated.

The landslide in the north slope of the highway might have been triggered by loss of toe support due to creek erosion. Highway ditch runoff erosion (at the TRM/gabion intersections) may have also been contributing factors.

There is currently significant settlement observed overtop of the culvert at Site 2, in the form of dips and cracks in the highway. The subsidence and cracking could be the result of embankment fill settlement but might also be an indication that the slide movement could eventually retrogress towards the highway.

At Site 1, the slide roughly paralleling the creek and highway is a direct result of Creek erosion and resulting soil loss along the toe of the embankment. The slope will tend to flatten with time due to loss of cohesion in the clay fill embankment material. Therefore, the slide could also gradually enlarge into the south (eastbound) driving lane of the highway surface, and there are already indications of this based on the existing crack and dip in the highway. The smaller 10 m wide slide near the east end of this site may have been triggered by east ditch runoff erosion, and/or having a slope that is too steep for the composition material of the embankment. Ditch or surface erosion of the slope could also contribute to more rapid slope movements.

Recommendations: **$ Ballpark Cost**

In the short term, regular monitoring of these sites should be undertaken for enlargement. If any of the slides encroach into the shoulders of the highway, barricades should be erected and enhanced with warning signage until highway repairs are undertaken. A temporary detour upslope (to the north of the highway) may also be required depending on the extent of the highway affected.

Site 1:

In the medium term, the recommended repair for only the small, shallower slide near the east end of the site is to subexcavate the failed slide mass down to intact clay, and rebuild the slope with imported pitrun gravel. The new fill material should be placed and compacted in thin horizontal lifts, benched into the intact slope surface, possibly utilizing a gravel shear key to stabilize the slide area. Some of the more suitable excavated clay could be used to provide a covering layer overtop the gravel as the finished slope surface to shed runoff. A subdrain should be installed along the base of the slide excavation surface, to drain any surface/subsurface water that may enter the rehabilitated slide mass. The east ditch along the toe of this slide should be re-contoured and lined with either TRM or gabions. All existing topsoil should also be stripped and salvaged for replacement and seeded at completion of the repairs.

$0.1 Million

For the medium to long term, a short highway re-alignment around the affected highway may be feasible. In order to meet the existing bridge (~100 m to the west), it would not be able to be a significant detour, but could work if only a small portion of the highway was affected by a slide. Riprap could also be installed to mitigate future creek erosion.

$2 to $3 Million

Alternatively, for the long term, due to the anticipated slide depth extending down to creek level and the embankment height (~13 m), a pile wall is feasible at this site. It would need to be a tied back pile wall (multiple anchors/pile), and a detour would be required to the north of the highway during construction. Perhaps other measures may be required in addition to the tied back pile wall, such as offloading a portion of the downstream embankment, and/or lightweight fill replacement of soil. A minimum length of 100 m of pile wall would be required to span the slide length at this site. Riprap should also be installed to mitigate creek erosion.

$4 Million

Site 2:

In the short term, a potential repair for the small slide on the upstream (north) embankment is to subexcavate the failed slide mass, and re-build the slope entirely with moisture conditioned (dried) clay reinforced by geogrid. This option would require a laydown/preparation area in order to re-use and moisture condition the clay. Additional riprap protection around the existing bridge culvert inlet would have to be placed. The existing gabion stone along the runoff ditches should be salvaged and re-instated over non-woven geotextile along new contoured ditches beyond the new repaired slope area. A suitable transition should be installed between the gabions and the erosion control soil covering further upslope (such as a steel plate and extended underlays, see PH64 for details), or alternatively (and preferably) consideration should be given to running gabions up the entire ditch slopes. Topsoil salvage will also be required.

$0.4 Million

For the medium to long term, due to the anticipated slide depth extending down to creek level and the embankment height (~16 m), a pile wall is likely not feasible at this site. A potential long-term solution for the south embankment slope is a highway re-alignment to the north, done in conjunction with the re-alignment for Site 1, before meeting the existing bridge over Hines Creek. A re-alignment could allow some flattening of the south slope leading down to the creek. For this option, the top section of the culvert might need to be re-profiled to remain within the embankment fill and the inlet end of the pipe would need to be extended to allow a shift and flattening of the north embankment slope.

$6 Million

As a minimum recommendation, a test hole drilled on the highway at each site to establish the soil and groundwater conditions is required, in addition to two more test holes complete with piezometer and slope inclinometer installations on the south embankment of Site 2.

$120,000

Additional test hole drilling would be required if a highway realignment or a pile wall is considered. The

culvert pipe should also be inspected for further signs of distress, and the locations of any distress should be surveyed. A topographic survey, detailed design and tender package will also be required prior to carrying out the remedial measures.