ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION-GRANDE PRAIRIE 2019 CALL OUT



Site Number	Location	Name		Hwy	km
Call Out GP16	Hwy 666:02	Mid-Valley/Old Major	(Site 1)		34+773
	S. of Grande Prairie	Lower Valley	(Site 2)	666:02	34+837
	(Wapiti River Valley Slides)	Valley Top	(Site 3)		34+250
Legal Description (Site 1)		UTM Co-ordinates (Site 1)			
NE¼ 15-070-06 W6M		11U E 383370	N 6103058		

	Date	PF	CF	Total	
Previous Inspection:	revious Inspection:		6	78 (Site 1)	
	29-May-2019	12	6	72 (Site 2)	
		13	5	65 (Site 3)	
Current Inspection:		13	6	78 (Site 1)	
	30-Aug-2019	14	6	84 (Site 2)	
		14	5	70 (Site 3)	
Road AADT:	268	30	Year:	2018	
Inspected by:	Ed Szmata, TRANS Dwayne Loewen, TRANS Roger Skirrow, TRANS		Nicole Wilder, Thurber Don Proudfoot, Thurber		
Report Attachments:	✓ Photographs✓ Plans✓ Maintenance Items		ce Items		

Primary Site Issue:	Three landslide features are affecting the emba 666:02 along the Wapiti River Valley south slope. A was evident since the last annual inspection.		
Dimensions:	Site 1: 300 m wide backscarp, (65 m wide at guardrail) and extends to the centerline. Landslide depth is assessed to be about 20 m near the roadway. Site 2: 300 m wide backscarp at guardrail, extending 300 m in length to the river valley floor with backscarp extending across roadway into the EBL ditch. Landslide depth is assessed to be at about 10 m depth along the roadway and about 20 m in depth at 150 m downslope towards the river. Site 3: 250 m wide backscarp and extends 1.5 m past edge of shoulder. The Landslide depth is assessed to be about 7 m to 11 m at the roadway.		
Maintenance:	Patching was done in June 2019 at all three sites. Milling of ACP along scarp drops in roadway and ACP patching of dips typically has been done every two years.		
Observations:	Description	Worsened?	
✓ Pavement Distress	Cracks and drops in pavement along the landslide backscarps are showing signs of significant activity since the last inspection in May of 2019. The cracks have shown through the recent patching in the same locations as observed in previous inspections and have increased in width and drop in some locations. An 80 mm thick ACP patch was placed at Site 1 and there is now a 150 mm wide graben with a 40 mm drop.	V	
✓ Slope Movement	Significant increases in the drops were observed along the backscarps at Sites 1, 2 and 3 as they	>	

Client: Alberta Transportation

File No.: 13353

E File: \\\\\H\\13353\\\2019 GP16 Inspection Report

		have been patched over and are still deeper and		
		wider than previously noted with increased rate		
		of movement. At Site 3 the backscarp has		
		continued to retrogress, the new ACP patch is		
		failing, and the scarp is now 200 mm past the		
		white pavement edge line of the WBL.		
- ·		Several sections of the EBL ditch bottom are	-	
✓ Erosion		eroded outside of the area that was protected	▽	
		with TRM at Site 3 which continues to worsen.		
✓ Seepage		Springs were noted in the EBL ditch at Site 1, 2 and Site 3 and were wet during the recent	~	
		inspection.	•	
		The inlet to a centerline culvert at Site 3 is still		
		obstructed by silt.		
☑ Bridge/Culvert	Distress	Erosion of the valley slope below centerline	~	
bridge/Odivert	Distress	culvert outlets were not inspected; however, are		
		likely in similar or slightly worse condition.		
□ Other		, , , , , , , , , , , , , , , , , , , ,		
L Other				
Instrumentation: (fr	om the Spr	ing 2019 Geohazard Instrument Readings)		
SI-1	Installed a	t the crest of the Wapiti River Valley slope in the	Landry Heights	
SI-2	residential	subdivision between Site 1 and Site 3. No discerna	able movement.	
SI-13		lownslope of the highway embankment within the	•	
01-10		e (Site 1). Showed a rate of 8.6 mm/yr over 1.3 to 5		
SI-41		n the upslope ditch at the Lower Valley (Site	2). Showed no	
		e movement since last readings.		
		ear the toe of the Wapiti River Valley slope below the Lower Valley		
		neared after the fall of 2015 at depth of 18.9 m.		
		ear the toe of the Wapiti River Valley slope below the Lower Valley		
	(Site 2). Showed a rate of movement of 27.8 mm/yr over 3 m to 5.5 m depth. Installed below the highway embankment at the Mid-Valley/Old Major Slide			
PN-13A/B	(Site 1). PN-13A showed a decrease in the water level of 0.06 m while PN-13B			
114 10/10	showed an increase in water level of 0.30 m since the fall 2018 readings.			
		near the toe of the Wapiti River Valley slope b		
DN 54 A /D	Valley/Adjacent Recent Slide (Site2). PN-51A showed an increase in the water			
PN-51A/B		13 m while PN-51B showed an increase of 0.23 m.		
PN-52A/B		change in water levels while PN-52B showed inc		
	levels of 0.	.11 m since the fall 2018 readings.		
		wing piezometers were installed downslope o	of the highway	
	embankme	ent in 2014 in the Valley top (Site 3) slide bowl.		
DNI4 4 54	Showed no	o change in water level.		
PN14-51 PN14-52		o change in water level.		
PN14-52		-		
VW14-55A	Showed a	n increase in water level of 0.78 m.		
	Choused =	o increase in water level of 0.47		
SP14-51		an increase in water level of 0.47 m.		
SP14-54	Showed at	n increase in water level of 0.36 m.		
Assessment:				

Assessment:

Conditions at all the 3 the sites have worsened since the last inspection in May of the same year. Springs were noted in the bottom of the EBL ditch at Sites 1, 2 and 3. Old cracks have shown through the series of new ACP patches that were done in June 2019 and are deeper and wider than previously observed. From discussions with AT, approximately 200 tons of asphalt was placed on June 17, 2019 and the road began to deform at the end of July and the scarp cracks showed through the new patches

Client: Alberta Transportation Date: August 30, 2019
File No.: 13353 Page 2 of 4

E File: \\H\13353\2019 GP16 Inspection Report

middle of August. At Site 1 there is active backslope movement as well as existing scarp cracks have shown through recent ACP patch and have worsened from recent movement throughout. A graben is present near the middle of Site 1. At Site 2, the drop off next to the highway on the west side is extremely steep and the highway is now above the guardrail from several ACP patches and the crack in the highway adjacent to SI-41 has worsened and is 20 mm wide and has a 70 mm drop. The cracks along Site 2 have generally worsened since the last inspection. At Site 3 the backscarp has actively retrogressed into the recent ACP patch and is now past the white pavement edge line and has a barricade placed as warning. In general, all three site have more differential settlements from the increased movement of the backscarps which poses greater hazard to the highway traffic.

Landslides are currently affecting the alignment of Hwy 666:02 at the following main locations:

Mid-Valley/" Old Major" Landslide (Site 1):

Deep seated landslide, believed to be rotational in nature at a depth of about 20 m to 25 m along the embankment sideslope, which corresponds to the top of the underlying bedrock and is affecting about 300 m of roadway. The toe of the landslide is believed to be at the bottom of the valley possibly within the limits of the Wapiti River bank itself. In addition, the backslope above the highway, which was offloaded and regraded in 1997-1998, has surficial slumps features above and below the intermediate level bench, which periodically cause slide debris to accumulate at the EBL ditch thus blocking the surface water flow.

Lower Valley Landslide (Site 2):

Deep seated rotational landslide assessed to be at 12m in depth along the embankment sideslope and 18 m in depth at mid slope and at 6 m in depth at the toe which is believed to be about 300 m downslope of the highway embankment, resulting in about 300 m of affected roadway.

Valley Top Landslide (Site 3):

Located near the top of the valley slope, assessed to be at about 7 m to 11 m depth along the highway embankment sideslope and is affecting about 250 m of roadway along the WBL shoulder.

Historically, except for the Valley Top Landslide (Site 3) from 2015 to 2016, these landslides have been relatively dormant or slowly creeping and as a result the affected sections of roadway have been maintained by milling and pavement overlays which are performed about every two years. However, the rate of movement at all three sites has increased this summer, likely in response to a period of prolonged rainfall.

Permanent localized in-situ stabilization remedial measures allowing to maintain the current roadway alignment, such as cantilever pile walls or soil anchor supported concrete piles are typically limited to landslide depths in the order of 10 to 15 m as stand alone structures. Consideration could be given to tiered level retaining wall systems and shifting the highway into the backslope and cutting it back; however, a geotechnical investigation would be required to further explore the feasibility of this scenario.

Mid to long term remediation scenarios would necessarily involve realignment or partial realignments which would necessitate excavation/off-loading and flattening of the backslope possibly requiring the purchase of land in the residential subdivision at crest of the valley.

A combination of realignment, offloading of the backslope and stabilization at the toe of backslope to minimize the footprint of the offloading excavation activities may also provide a midterm remedial solution and would also require a geotechnical investigation to be further assessed.

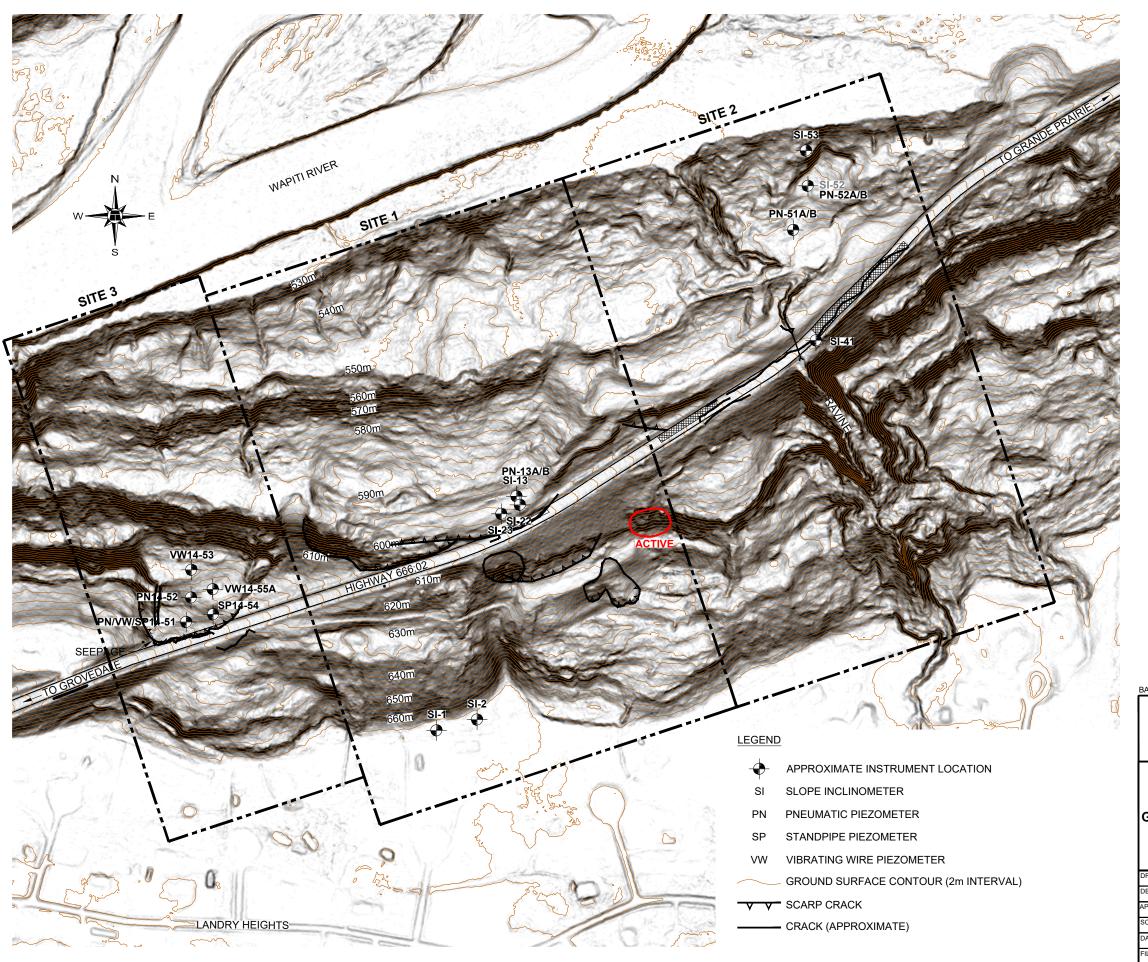
All the remediation alternatives involve either costly regular maintenance measures or elaborate stabilization techniques. Due to the light usage (2680 AADT) a more pragmatic solution in the long term might be to close this section of the highway, or at least reduce its service rating to a secondary or emergency access road, implement gravel surfacing to avoid costly asphalt surface repairs and to provide access to the area either via Grovedale or a new access road connecting to Hwy 40 to the east from the upper plateau.

Client: Alberta Transportation Date: August 30, 2019
File No.: 13353 Page 3 of 4

E File: \\H\13353\2019 GP16 Inspection Report

Recommendations:	Ballpark Cost
In the short term, it is recommended that the cracks in the pavement be sealed or overlain with an asphalt patch, that the drops along the backscarps be milled and that the area be closely monitored for future signs of movement.	Maintenance
The ditch block at the inlet and outlet to the centerline culvert at Site 3 should be removed.	
The inlet of the centerline culvert at Site 2 should be cleaned out. The outlet should be re-established, and the channel lined with rip-rap with a dissipation bowl structure.	
The guardrail should be extended in the critical areas and installed above the highway as it is currently below due to several ACP patches. Traffic should also be slowed to 50 km/hr and signs for uneven pavement and sharp shoulders for 2 km should be installed to warn public.	
A geotechnical investigation is required to assess the mechanisms of failure and to design repair measures for each of these landslide sites. A comparison of LIDAR from different years should also be performed. Due to the history of activity at these sites, it is recommended that the geotechnical investigation be implemented as soon as possible. Some economy could be made if all 3 sites were assessed by the same field investigation under a single mobilization of drilling equipment.	\$60,000 per site or \$150,000 (for all 3 sites)
In the mid term, a partial realignment at Site 1 could be performed with the support of a pile wall. The realignment alone would provide about 5 years of additional service life to the current alignment through the site. There is a possibility to extend the life span to about 10 years with the additional support of a pile wall on the upslope side.	\$5,000,000 (Realignment Only) \$10,000,000 (Realignment with Pile Wall)
For Site 3, a pile wall could be constructed to help preserve the current alignment for about 5 to 10 years.	\$3,000,000 to \$5,000,000
For Site 2, further geotechnical investigation would be required to propose an effective mid-term solution such as pile walls or realignment through this site. Consideration should be given to replacing the current 600 mm diameter SWSP culvert with a larger diameter pipe connected to a solid drain pipe that would extend down the slope to the valley floor.	
The long-term solution for the section of the roadway affected by the current landslide features would be a major realignment.	\$50,000,000

Alberta Transportation 13353 \\<u>H\13353\2019 GP16 Inspection Report</u> Client: File No.: E File: Date: August 30, 2019 Page 4 of 4

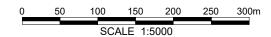




SITE MAP NOT TO SCALE

NOTES:

- 1. FEATURE LOCATIONS ARE APPROXIMATE.
- 2. PREVIOUS OBSERVATIONS SHOWN IN BLACK.
- 3. AUGUST 30, 2019 FEATURES SHOWN IN RED.



BASE PLAN PROVIDED BY ALBERTA TRANSPORTATION



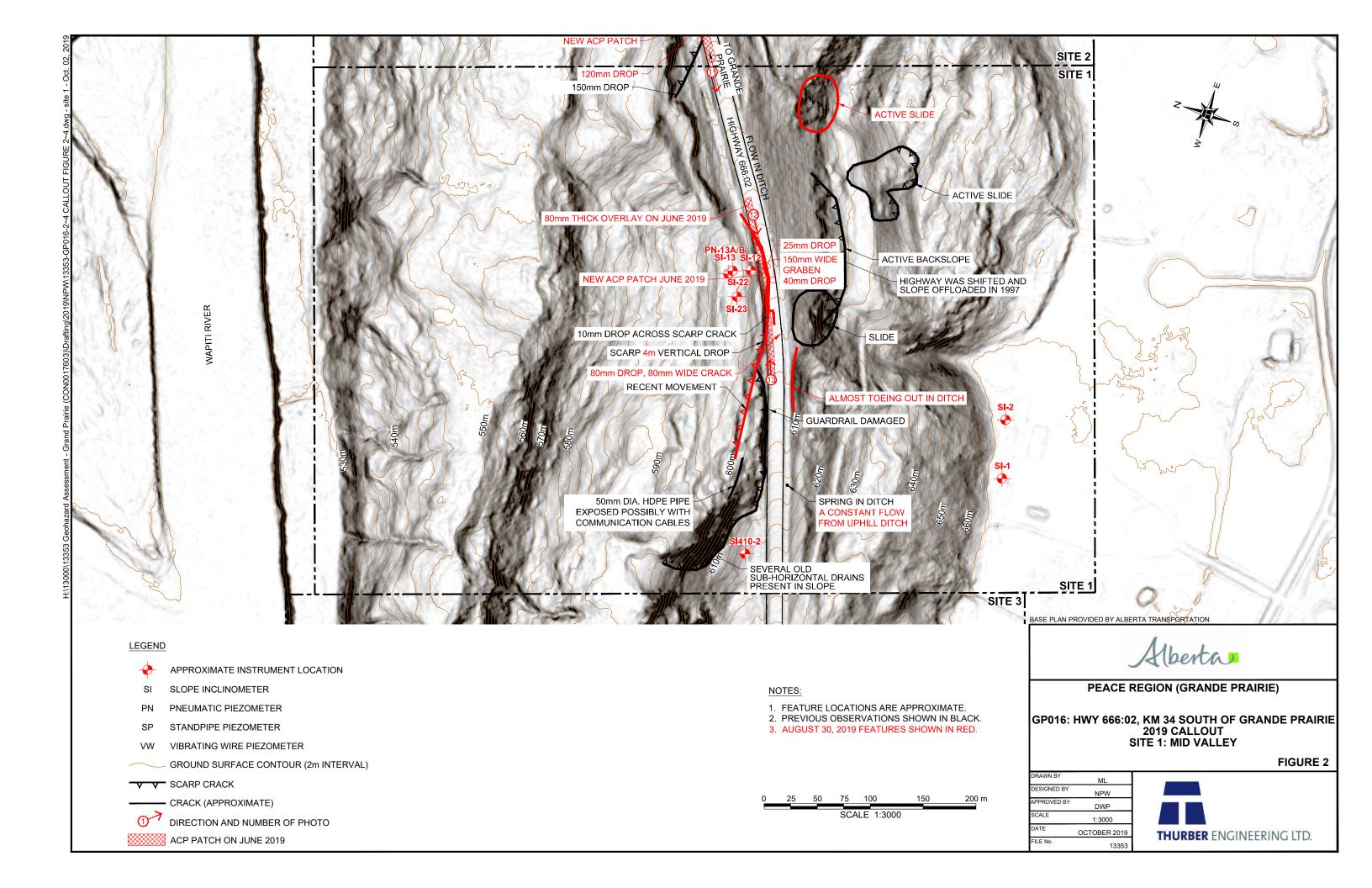
PEACE REGION (GRANDE PRAIRIE)

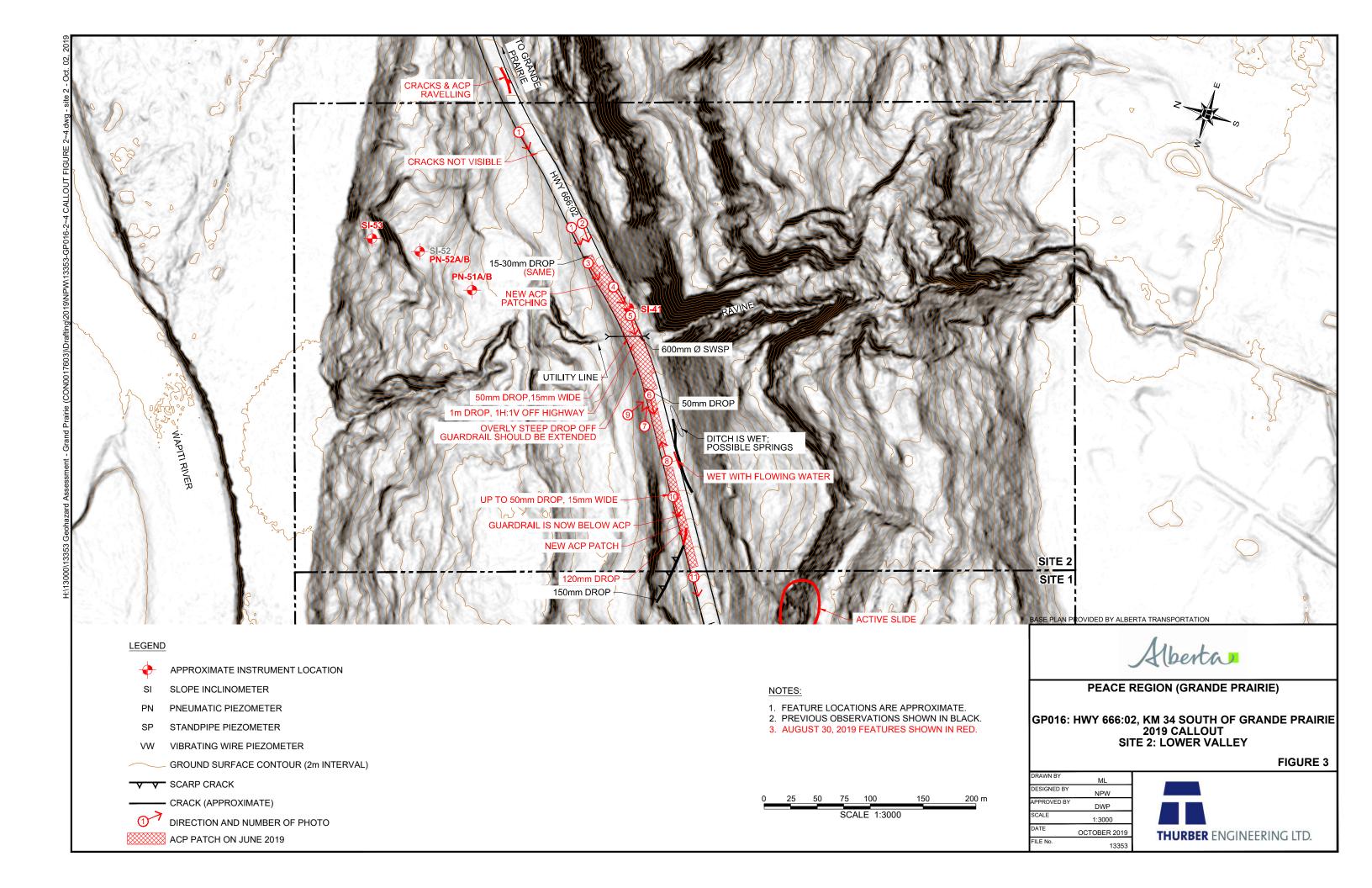
GP016: HWY 666:02, KM 34 SOUTH OF GRANDE PRAIRIE 2019 CALL OUT

FIGURE 1

DRAWN BY	ML
DESIGNED BY	NPW
APPROVED BY	DWP
SCALE	1:5000
DATE	SEPTEMBER 201
FILE No.	1334







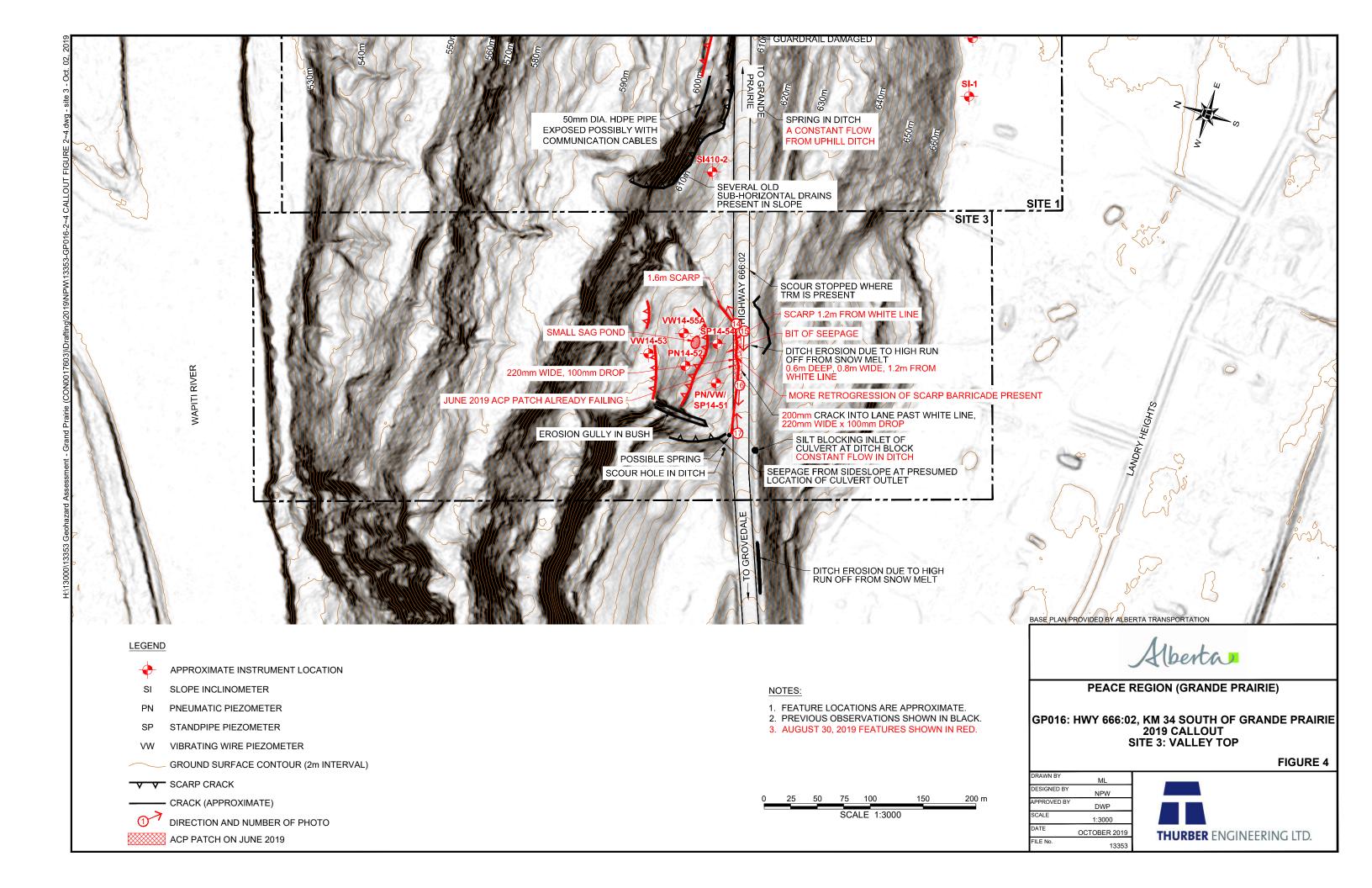






Photo 1. Looking southwest towards the Lower Valley Landslide (Site 2).



Photo 2. Looking southwest towards the Lower Valley Landslide (Site 2).



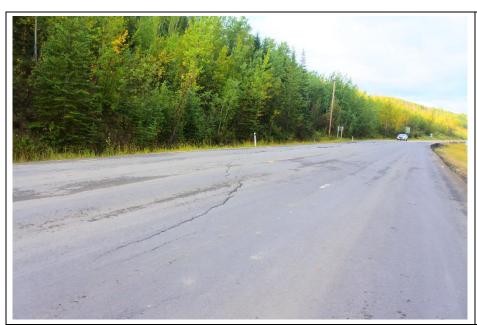


Photo 3.
Looking southwest along Hwy 666 at west portion Lower Valley Landslide (Site 2). The drops along the scarp in the ACP in this section in the roadway surafce were about 15 mm to 30 mm.



Photo 4. Looking southwest along Hwy 666 at middle portion Lower Valley Landslide (Site 2). The drop along the scarp in the ACP in this section in the roadway surafce was up to about 70 mm





Photo 5.
Looking southwest along Hwy 666 at west portion Lower Valley Landslide (Site 2). The scarp has a 0.3 m drop in the WBL shoulder.



Photo 6.
Looking southwest along Hwy 666 at west portion Lower Valley Landslide (Site 2). The scarp has a 0.3 m drop in the WBL shoulder.





Photo 7.
Looking northeast along cracks in pavement at Lower Valley Landslide (Site 2) that extend off the roadway.



Photo 8. Looking northeast along cracks in pavement Lower Valley Landslide (Site 2).





Photo 9.
Looking east at overly steep drop off from several layers of ACP patching at Lower Valley Landslide (Site 2).



Photo 10.
Looking southwest along WBL between the Lower Valley Landslide (Site 2) and the Mid-Valley/"Old Major" Landslide (Site1). Cracks drop from 50 mm to 120 mm along the scarp at the west end.





Photo 11.
Looking southwest from the WBL shoulder at the slide in the backslope in the eastern portion off the Mid-Valley/"Old Major" Landslide (Site 1).



Photo 12.
Looking west at the ACP patch in outer WBL at Mid-Valley/"Old Major" Landslide (Site 1). There is a 40 mm drop along the scarp in the ACP.





Photo 13.

Looking northeast at graben below the highway at the Mid-Valley Landslide / "Old Major" (Site 1) and the cracking in the ACP in the WBL. The drop in the ACP along the backscarp near the guardrail at the west end is now at about 80 mm and the cracks are open to 80 mm.



Photo 14.

Looking northeast from the shoulder of the WBL at the scarp extending into the brush at the Valley Top Landslide (Site 3).





Photo 15.
Looking southwest along backscarp of Valley Top
Landslide (Site 3).
Backscarp drop in embankment side slope now about 1.5 m



Photo 16. Looking southwest along backscarp in the west section of the Valley Top Landslide (Site 3). Site was patched in June 2019. Cracks extend 200 mm past the white line into the WBL, are open 220 mm and are 1 m deep.





Photo 17.
Looking northeast along backscarp in the west section of the Valley Top Landslide (Site 3).