

GEOHAZARD RISK MANAGEMENT PROGRAM

North Central Region – Edson Area



2013 Inspection Report

Site Number	Site Name		Hwy		km	
NC15	Pile Retaining Wall (North of Edson)		748:02		6.2	
Legal Land Description		UTM Coordinates (NAD83)				
NW10-54-17-W5M		Zone 11U	N5945148 E53		537511	
Operational Site Instrumentation		Slope Inclinometers			0	
		Pneumatic Piezometers			0	
		Vibrating Wire Piezometers		5	0	
		Standpipe Piezometers			0	
Date of Last Inst	rumentation Readings	mentation Readings N/A				

	Date	PF	CF	Rating	
Current Inspection	June 26, 2013	8	3	24	
Previous Inspection	May 17, 2011	8	3	24	
Report Attachments	Photographs (7 pages)	Site Plans (1 page)			

	Golder	Alber	ta T	ranspor	tation
Inspected By	Ian Darrach; Eric Leishman	Roger Skirro	w; Ho	oward Haw	/ley
Date of Remediation	Pile wall and gabion baskets installed in 1	999			
Recent Maintenance	Asphalt patch and seal coat applied in 20	12 due to poo	or ride	e quality	
Primary Site Issue	Embankment slumping below the highway	y, likely due to	o hig	h groundw	ater levels
Observations	Description and Location		Change From Previous Inspection		
Pavement Distress	Crack reflecting through patch		\square	Yes	No No
Culvert Distress				Yes	🗌 No
Bridge Distress				Yes	🗌 No
Slope Movement	Fresh movement observed on/near existing	ng scarps	\square	Yes	🗌 No
Erosion	Minor erosion on west side of north abutn	nent	\square	Yes	🗌 No
🛛 Seepage	Spring, moss growth at base of gabian ba	isket	\square	Yes	No No
Other				Yes	🗌 No



Discussion	The crack in the south patch (see Figure 1) has started to reflect through the patch that was applied in 2012. Currently, the crack is less than 5 mm in width with 0 mm differential. The crack begins in the shoulder of the west lane and extends through the first third of the SBL, as shown in Photo 1. The crack appears to be taking the same shape as the previous crack.
	New slope movement was observed near the existing scarps to the south of the pile wall. Tension cracks and freshly exposed earth were found in these areas, in addition to soft, saturated ground and leaning trees (see Photos 2 to 4). Further, the ground surface was hummocky below the scarps, as the slope gradually bulges towards the river, as noted in Photo 5. No new movement was observed in the scarp below the pile walls.
	A subsidence of approximately 600 mm in height was observed in the upper gabion wall. A hole in the gabion wall was also observed at this location, as shown in Photo 6, as well as a spring and moss growth.
	Surface erosion and seepage were observed on the west side of the north bridge abutment, as shown in Photos 7 and 8. The erosion gulley observed was approximately 800 mm wide, between 100 and 150 mm deep and about 12 m long. Photo 8 shows seepage flowing into the south end of the gulley.
	The culvert on the east side of the north abutment was replaced in 2012 with a half round culvert, anchored by steel pins embedded into the ground (see Photo 9). The ground surface beneath the coconut matting on either side of the culvert was observed to be soft and saturated. The ground surface in the east ditch was found to be saturated as well, from the bridge to the intersection to the north. Standing water was observed in this ditch approximately 100 m south of the intersection, as seen in Photo 10.
Assessment	Based on the above observations, the pile walls constructed in 1999 appear to be performing satisfactorily. Some seepage was noted through the upper gabion wall, at the location of the hole in the gabian basket; however, this does not appear to be affecting the performance of the wall.
	Although the crack to the south of the pile walls has reflected through the patching, there is no vertical differential at this time. The fresh slope movement observed near the existing scarps downslope of this crack is likely due to higher water levels (both groundwater and the river level) resulting from increased precipitation over the last two years. There is the potential for future regression of the scarps to the pavement surface, as well as river erosion of the toe of the slope.
	Short term recommendations include:
Recommendations	 Sealing pavement cracks as necessary to reduce surface water infiltration into the slope; Regular observations of the pavement crack to the south of the pile wall; and, Repairing the rutting on the west embankment slope to reduce the potential for ponding water.
	slope should be considered to reduce the groundwater level within the embankment. The drains could consist of trenches filled with free draining gravel wrapped in a geotextile, and should be daylighted downslope.
	The crack to the south of the retaining wall should be monitored on a regular basis for signs of further distress. It is recommended that inspections be conducted annually. Instrumentation should be considered if further distress is observed in the pavement.







NC15 - PILE RETAINING WALL (NORTH OF EDSON) 2013 Annual Inspection Photos



PHOTO 1: Pavement crack south of retaining wall reflecting through patch, less than 5 mm wide.





PHOTO 2: Freshly exposed earth in scarps south of pile wall.







PHOTO 3: Leaning tree at toe of slope, south of pile wall.







PHOTO 4: Tension cracks in slope below crack south of pile wall.



PHOTO 5: Hummocky terrain/toe bulge, south of pile wall.





PHOTO 6: Hole in upper gabion wall.



PHOTO 7: Erosion gulley, west side of north bridge abutment.





PHOTO 8: South end of erosion gulley, west side of north bridge abutment. Note seepage and standing water.



PHOTO 9: Half round culvert installed in 2012.





PHOTO 10: Standing water in east ditch, ~100 m south of intersection; facing north.

