GEOHAZARD ASSESSMENT PROGRAM

NORTH CENTRAL REGION – ATHABASCA

2017 INSPECTION



Site Number	Location		Name		Hwy	Km
NC 014-1	Northeast boundary of the Town of Fort Assiniboine		Fort Assiniboine		661:02	1.8
Legal Description	on		UTM Co-ordinates (NAD 83)			
NW-1-62-6-W5M			11 N 6023391		E 644779	
		Date	PF	CF		Total
Previous Inspe	ction:	May 18, 2016	8	4	32	
Current Inspec	tion:	May 17, 2017	8	4		32
Road AADT:		1000		Year:	2016	
Inspected By:		Tarek Abdelaziz, José Pineda (Thurber) Roger Skirrow, Arthur Kavulok (TRANS)				
Report Attachn	nents:	Photographs Plans Daintenance Iten			tenance Items	

Primary Site Issue:	Slope creep movements causing pavement distress to a side hill alignment due to seasonal high ground water levels	
Dimensions:	About 250 m long	
Remediation	None recently	
Maintenance:	 Silt in Manholes 1 and 2 was hydrovacced from the interior of the CSP pipes. Highway cracks were spray patched in the fall 2014. Mr. Dale Kluin of the County Council provided the following information during the 2017 site visit: Slope stability issues occurred during the construction of the highway in the 70's. Two subdrain pipes were installed in the highway east ditch to reduce groundwater levels in the sandy soils. The subdrains were installed approximately 2.4 m below the ditch grade. The subdrain system extends from Manhole MH4 located near the top of the hill to the existing creek near the bottom of the hill. After the installation of the subdrain system, icing issues were noted in the highway ditch and its surface. The icing issue was believed to be in response to uncontrolled surface flow from the hill side towards the highway alignment. Two manholes were constructed in the highway backslope to collect surface water from springs/natural drainage gullies. Each of the manholes through a trench drain filled with rock. A trench drain filled with rock was also used to convey the flow from the vertical CSP drop pipe to the subdrain system in the ditch. Visual inspection of the drainage system during the 2017 site visit indicates the subdrain system consists of two 150 mm HDPE perforated pipes as observed from the inside of MH5 (cleanout manhole) and at the subdrain outlet locations. 	

	Description	Worse?
Pavement Distress	40 to 50 mm depression in the SBL of the Mid-Hill section; slight dip in the highway southbound lane behind the graben feature	•
Slope Movement	Mid-Hill slope section: 40 to 90 mm wide cracks with up to 40 mm differential height across cracks;	
Erosion		
I Seepage	 MH#1: Water level measurement did not vary since 2016 MH#2: Heavily oxidized interior walls. Minor flow from one of the sub-horizontal drains Subdrain: Water was flowing heavily at the subdrain outlet locations 	
Bridge/Culvert Distress		
Other Instrumentation: (2SIs and 11	600 mm long x 300 mm wide 200 mm deep sinkhole in the highway ditch; 2.5 m long x 1.5 m wide x 0.2 m deep drop/settlement near the edge of the road; sag in the guardrail behind the graben feature located downslope of the highway to the north of the Mid-Hill section	
No discernable movement in SI by +/-0.4 m; SP06-1 showed a SP06-19 continued to be dry. The site observations and histo	06-12 has been blocked since 2011. 06-6 and 06-11; groundwater levels fluctuated in most of th a decrease of 0.91 m and SP06-19-5 showed an increase prical instrument readings indicate that the slopes outside xperience slow creep movements due to seasonal fluctuation	e of 0.84 m;
		ons in ground
	nued to be active and is impacting the highway condition. The section created a rough ride to motorists.	-
distress within the Mill-Hill slope The local landslide located dow		he pavement
distress within the Mill-Hill slope The local landslide located dow to be dormant. The existing dip reflection of an old movement. The surface and subsurface d during construction (i.e. subdra highway surface) appear to ha	section created a rough ride to motorists. nslope of the highway to the north of the Mill-Hill slope sec in the highway SBL and the noted sag in the guardrail ar rainage improvement measures installed above the high ain pipes in the east ditch and CSP pipes and manhole we reduced groundwater levels and improved the stabilit nprovement measures will likely need to be installed to fu	he pavement stion appears e probably a way surface s above the y of the hill.
distress within the Mill-Hill slope The local landslide located dow to be dormant. The existing dip reflection of an old movement. The surface and subsurface d during construction (i.e. subdra highway surface) appear to ha However, additional drainage ir groundwater levels and hence th The presence of clear water in I 2014. It is likely that the rock fille	section created a rough ride to motorists. nslope of the highway to the north of the Mill-Hill slope sec in the highway SBL and the noted sag in the guardrail ar rainage improvement measures installed above the high ain pipes in the east ditch and CSP pipes and manhole we reduced groundwater levels and improved the stabilit nprovement measures will likely need to be installed to fu	he pavement stion appears e probably a way surface s above the y of the hill. inther reduce completed in

Recommendations:

In the short term, it is recommended to seal all open cracks in the pavement to prevent surface water infiltration into the landslide cracks. A bump sign should be placed to the north of the Mid-Hill section dip area to warn motorists of the existing hazard. Consideration should be given for patching the Mid-Hill slope section to provide a smooth ride to travellers on the highway.

The existing sink hole in the ditch and the drop by the edge of the roadway should be backfilled with either gravel or clay.

In the long term, additional drainage improvement measures, such as deep drainage wells, will need to be installed above the highway to reduce ground water levels and further enhance the slope stability at this site. Detailed site survey and field slug tests were conducted as part of the preliminary engineering work that is being conducted to address ongoing movements at this site. Detailed long term remedial measures will be provided in the preliminary engineering report.