ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM NORTH CENTRAL REGION – ATHABASCA AND FORT MCMURRAY DRISTRICTS 2021 SITE INSPECTION



Site Number	Location		Name		Hwy	km		
NC104	Approxima of Hwy 29 (North of E	ately 7.7 km north Elk Point)	KEHIWIN	LAKE	41:23	7.7		
Legal Description			UTM Co-o	ordinates (N/	AD 83)			
NE-25-58-7-W4M 12 N 5988325 E 506652								
		Date	PF	CF	Tot	tal		
Previous Inspection:		June 12, 2019	8	4	32			
Current Inspection:		June 25, 2021	11	4	44	44		
Road AADT:		1,230)	Year:	2020			
Inspected By:		José Pineda, Tarek Abdelaziz (Thurber) Kristen Tappenden, Bernard Ching (Alberta Transportation)						
Report Attachments:		Photograph 🗹	Photographs 🛛 Plans			Maintenance Items		
Primary Former Site Issue:		Landslide (N the south of NC24A) afte pavement dis	Landslide (NC104, previously known as NC24C) developed abruptly to the south of the km 7.8 pile wall (NC103 site previously known as NC24A) after occurrence of heavy rainfall events in 2011, causing pavement distress in the highway surface.					
Dimensions:		Cracked section of highway to the south of the southern limit of the NC103 site: About 180 m long (parallel to the highway alignment) and 80 m wide (parallel to the slope direction). Area susceptible to future local landslides to the south of the cracked section of the highway: About 160 m long.						
Date of any remediation:		fall of 2017 and consisted of the following: (a) the construction of a 230 m long cast-in-place tied-back pile wall to the south of NC24A to retain the cracked section of the highway (NC24C tied-back wall), (b) the construction of a 125 m cast-in-place cantilever pile wall (NC24 pile wall extension) to the south of the NC24C tied-back wall as a precautionary measure against future landslides. The NC24C pile wall extension (NC24C interim pile wall) was designed to accommodate the installation of additional piles, anchors and waler (if needed) in the future subject to instrumentation monitoring results, (c) installation of twin culverts and lining of C2 culvert by others, (d) grouting of culvert C1, ditch regarding and lining with TRM. Slope inclinometers load cells were installed in the NC24C tied-back pile wall to monitor the wall movement and anchor loads. Slope inclinometers, a Shape Accelerometer Array (SAA) and strain gauges were installed in selected piles of the NC24C pile wall extension to assess wall movements and the bending moment in one of the piles.						
Maintenance:		fall of 2011; pronounced of fa	distressed a oatched agai pring of 2013 ed after cons	was complete rea of the high n in fall 2012; 3; cracks were truction in the	eu on may 12, 2 nway surface was cracks opened ag sealed in June 20 fall of 2017.	patched in the jain and sealed)14. ACP patch		

Observations:	Description	Worse?			
Pavement Distress	N/A				
Slope Movement	N/A				
Erosion					
Seepage					
Bridge/Culvert Distress					
Other	Sharp drop off on the west side of highway due to previous ACP patches; cattails in the south ditch				
Instrumentation: (1SAA, 16SIs, 7PNs, 5SPs, 16VCs, 18SGs)					
The following provides the slope inclinometers' rate of movements in the Spring of 2021:					
• SI15-15 (located in the east ditch within the southern limit of the landslide) = 0.7 mm/yr.					

- SI15-16 and SI15-21 (located near the west edge of the highway downslope of the pile walls) ranged between 0.1 and 1.0 mm/yr.
- SI12-11, SI15-7, SI15-20 (located at the bottom of the highway west side slope) ranged between 0.9 and 2.6 mm/yr.

Total pile head movement (between fall 2020 and spring 2021):

- N24C tied-back pile wall: SI16-1= -6.2 mm, SI16-2 = -4.7 mm, SI16-3 = -3.6 mm, SI16-4 = -4.3 mm, SI16-5 = -1.3 mm.
- N24C Interim pile wall: SI17-1= 3.2 mm, SI17-2 = 8.8 mm, SI17-3 = 15.5 mm, SI17-4 = 9.4 mm, SAA17-1 = 1.9 mm.

Vibrating wire load cell readings ranged from 179 kN to 256 kN; The readings of the strain gauge, installed in Pile 146 of the NC24C Interim pile wall, are still below warning threshold values.

Groundwater levels fluctuated in the piezometers and ranged from an increase of 0.4 m in SP12-9 to a decrease of 0.5 m in PN15-16.

Assessment (Refer to attached Figure):

The site observations and instrumentation readings indicate that implemented remedial measure has been effective in stabilizing the landslide mass. Reflective highway surface cracks may however appear over time until the pile wall mobilizes the full magnitude of the stabilizing force.

At present, the four slope inclinometers installed in the interim pile wall show that the pile head deflections (3mm, 9 mm, 10 mm, 15.5 mm) are approaching the suggested control deflection value (i.e., 16 mm). The pile head deflection in one of the piles has already exceeded the warning threshold value (i.e., 12 mm). There is a concern regarding the integrity of the cantilever pile wall. If the piles move excessively or fail in the future, an abrupt movement of the landslide and hence the highway will take place.

The presence of cattails in the ditch may indicate poor surface drainage in the ditch, but this will need to be confirmed through further monitoring of the site. Poor drainage, and possibly water ponding, in the ditch can result in elevated groundwater levels and an accelerated landside movement at the interim pile wall location.

The existing sharp drop off near the edge of pavement still constitutes a safety hazard to runaway vehicles.

The probability of failure increased from 8 to 11 to account for potential excessive movement of the interim pile wall.

Recommendations:

This site should be visited again in the spring of 2023, as per the GRMP's schedule, assuming that the interim pile wall instrumentation are read semi-annually at this site. These instruments are currently read annually.

The local MCI should watch closely for the development of any cracks in the highway section above the pile wall extension and notify us immediately. Cracks should generally be sealed to reduce ground water infiltration into the landslide mass.

The local MCI should also check whether the south ditch is draining towards the C2 pipe and the twin culverts. If water is ponding within the ditch, consideration should be given to re-grading the ditch to promote positive drainage.

Consideration should be given for placing the least amount of fill off the highway surface to smoothen the existing sharp drop off and eliminate the existing hazard. Otherwise, sharp shoulders warning signs should be erected to warn the motorists of the existing hazard.

The installation of the full retaining wall system (i.e., anchors and waler) should be considered for the interim pile wall before the structural integrity of the piles is compromised.

Closure

It is a condition of this letter report that Thurber's performance of its professional services will be subject to the attached Statement of Limitations and Conditions.

Tarek Abdelaziz, Ph.D., P.Eng. Principal | Senior Geotechnical Engineer

José Pineda, M.Eng., P.Eng. Senior Geotechnical Engineer







Photo No. 1 - Looking north at highway surface; no reflective cracks or dips noted in 2021



Photo No.2 - Looking northeast at the twin culverts outlets; Sediment and silt fence cleared; no flow in 2021





Photo No.3 - Looking at the twin culverts inlets; no flow in 2021



Photo No.4 - Looking north at the TRM lined ditch; no water flowing; note cattails in the ditch area





Photo No.5 - Looking at culvert C2 inlet; no flow in 2021



Photo No.6 - Looking at data logger DT1 installed at pile 146 location; vegetation grew at previously noted subsidence at the base of the data logger





Photo No.7 – Looking north at sharp drop off shoulder on the west side slope



Photo No. 8 – Looking at gully lined with TRM; note lush vegetation but no flow in 2021





Photo No. 9 - Looking south at TRM lined ditch; note lush vegetation but no flow in 2021



Photo No. 10 - Looking north at the highway surface from the northern limit of the site