GEOHAZARD ASSESSMENT PROGRAM NORTH CENTRAL REGION – ATHABASCA 2017 INSPECTION



Site Number	Location	Hwy	Km	
NC 004-1	Town of Athabasca	55:10 and 813:02 Intersection	Hwy 55:10: km 0.34 to 0.84 Hwy 813:02: km 0.00 to 1.97	
Legal Description		UTM Co-ordinates (NAD 83)		
21-066-22 W4M		12 E 6066180	N 353385	

	Date	PF	CF	Total
Previous Inspection:	May 18, 2016	12	4	48
Current Inspection:	May 17, 2017	12	4	48
Road AADT:	9100 Y		Year:	2016
Inspected By:	Tarek Abdelaziz, José Pineda (Thurber) Roger Skirrow, Arthur Kavulok, Ron Hilligas, Paula Campbell (TRANS)			
Report Attachments:	☑ Photographs	☑ PI	ans	✓ Maintenance Items

Primary Site Issue:	Light to severe erosion of Hwy 55 highway side slopes/ditches, Hwy 813 south ditch, and the NE and NW corners of the Tawatinaw River bridge (BF1517-2) approach fill side slopes		
Dimensions:	Refer to attached drawings and notes below		
Maintenance:	May 2015: Hwy 55 south side slope: TRANS placed cold mix along the shoulder of the side slope to fill in erosion gully. Hwy 55 south ditch: TRANS placed Class 1M riprap along a short section of the ditch, and installed synthetic ditch barriers. TRANS placed an ACP patch on Hwy 55 to correct an area of settlement that had occurred over the former Tawatinaw River channel. September 2015: TRANS lined the eroded south ditch of Hwy 55 with Class 1 M riprap between Gabion weirs G4 and G9, backfilled the two 900 mm holes near existing light standards, lined the existing swale between 50 Ave and the south ditch of Hwy 55 with Class 1 M riprap, installed additional synthetic ditch barriers in Hwy 55 south ditch, and repaired the damaged guardrail section along the northwest corner of Hwy 813 and Hwy 55. September 2016: TRANS backfilled existing erosion gullies at the NE and the NW corners of the Bridge with gravel.		
Observations:	Description	Worse?	
☐ Pavement Distress	N/A		
☐ Slope Movement	N/A		
Slight to severe erosion gullies between the widely-spaced gabion weirs installed in the ditches of Hwy 55 and Hwy 813; rill erosion in the south and north side slopes of Hwy 55, along the side slope of the Hwy 813 tie-in to Hwy 55, the northwest side slope of the intersection of Hwy 55 and 43 St.; severe erosion gully along the NE corner of the bridge side slope; erosion beside and below the existing drain trough at the NW corner of the bridge; existing riprap-lined bowl at the outlet of the bridge NE corner drain trough is full of sediment			

Client: Alberta Transportation Date: September 6, 2017

□ Seepage	N/A	
☑ Bridge/Culvert Distress	Some fill settlement has occurred around the Hwy 55 south ditch manhole/drop pipe that drains the median ditch to the storm sewer pipe	
✓ Other	Slight damage in the sides of the Hwy 55 north ditch G14 and G17 wire baskets (a few rocks came out of the baskets); sections of the existing guardrail at the NE corner of the bridge area got damaged again; low area (filled with water) in the highway 55 south ditch to the west of the manhole/drop pipe	

Instrumentation: (1PN)

The slope inclinometers installed at this site were either sheared off prior to construction or damaged during the construction of the intersection improvement project.

The only operational pneumatic piezometer (PN04-2) located in the south ditch of Hwy 55 indicates that the groundwater level at this location is about 3.4 m below existing ground surface.

Assessment:

The site condition did not change significantly since the last site visit completed in 2016.

The erosion that has been occurring along the side slopes is due to concentrated runoff flowing over bare slopes in sandy soils. Hwy. 55 and Hwy. 813 have curved alignments at the areas of concern with super elevations that concentrate and directs runoff to the side slopes. The side slopes are generally bare of vegetation, possibly due to salting of the hill sections of Hwy 55 and Hwy 813.

The sandy soil, that is present in the side slope and median ditch bottoms, is highly susceptible to erosion when left in a bare, unprotected condition and exposed to concentrated runoff. Similar ground conditions have performed much better in other areas on site where runoff occurs as sheet flow rather than a concentrated flow.

The existing gabion type rock weirs installed in the bottom of the Hwy. 55/50 Ave. median ditch are spaced quite far apart (about 25 to 30 m). The ditch gradient is relatively steep (approximately 6 to 8 percent) and the existing weirs should have been placed at a closer spacing to prevent erosion from taking place. Otherwise, heavy armouring of the ditch bottom should have been provided between the widely-spaced weirs. The erosion control blanket placed between the weirs provided a temporary erosion control measure and a more robust permanent erosion control measure should have been considered to provide a long-term protection against erosion.

The median ditch between Hwy. 813 and Hwy. 55 has a steep gradient near the top of the hill (inclined at approximately 9 percent); however, the closely spaced weirs, which are about 7 m apart, performed relatively well with little to no significant signs of erosion between the weirs below the ECB. Severe erosion was however noted between the 25 to 30 m apart weirs within the flatter gradient of the ditch (inclined at approximately 5 to 6 percent) near the bottom of the hill.

A few of the existing riprap channels in the south ditch of Hwy 55 are not well defined (i.e. appears to be flat) with no riprap along the ditch slopes and this could result in severe erosion developing near the edges of the riprap. The swale between 50 Ave and Hwy 55 south ditch was not provided with adequate thickness of Class 1 M riprap in addition to the presence of bare spots between the rocks. This deficiency could result in future erosion issues within the bottom and the sides of the swale.

The concentrated runoff occurring at the northwest corner of the intersection of Hwy 55 and Hwy 813 (i.e. at the NE corner of the bridge) created 1 m deep and 1 m wide deep erosion gullies down the side slope. The existing erosion beside and below bridge NW corner drain trough indicates that the drain trough should have been designed with a wider taper to accommodate surface flow from the highway surface.

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File: 13357 e-file: \\H\13357 \NC004-1 The damage occurred in the wire mesh of the G14 and G17 baskets could be attributed to localized settlement of fill below the affected baskets resulting is excessive stretching and failure of the wires.

The top of the catch basin, located at the outlet of the bridge NW corner drain trough, is lower than the bottom of the riprap-lined dissipation bowl by at least 300 mm. Sand, salt and erosion debris accumulate within the bowl and fall into the catch basin. The catch basin, which is connected to the river through a 300 mm diameter outflow pipe, is likely full of sediment and needs to be cleaned to prevent the transport of sediment into the river.

Recommendations:

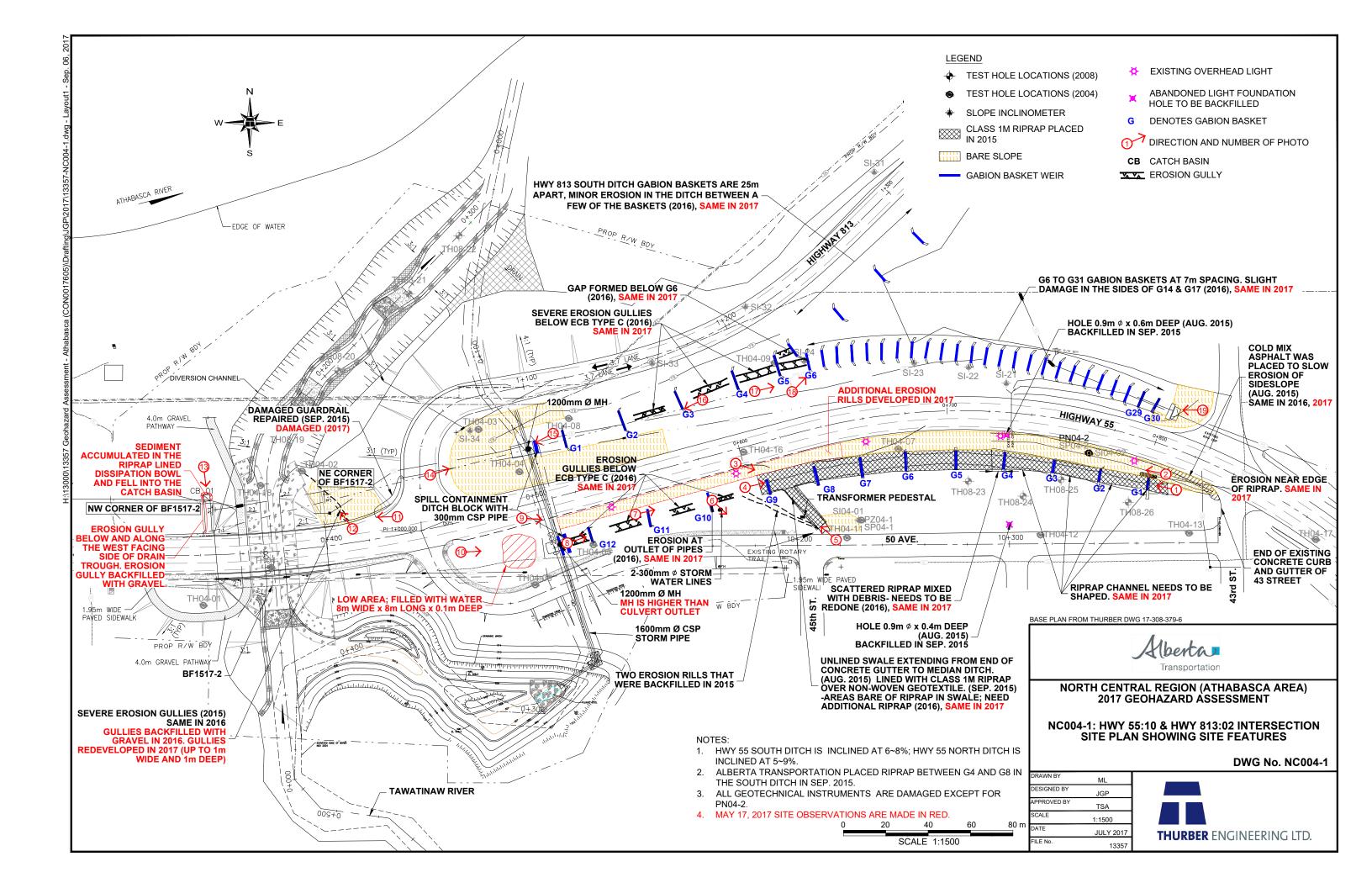
Thurber completed a detailed designed to address the erosion issues occurring at this site. Alberta Transportation has tendered and awarded this project and the construction is scheduled to be completed in the fall of 2017.

The issued for construction drawings are attached at the end of this report. In general, the repairs consist of the following:

- 1. South of HWY 55: The existing 150 mm thick Class 1M riprap in the ditch and the 50 Ave swale will be salvaged; erosion gullies in the ditch will be repaired by removing all debris and backfilling eroded surfaces with compacted clay; the ditch and swale will be slightly regraded to build well defined channels; the south ditch and the 50 Ave swale will be lined with 300 mm thick Class 1M riprap and/or Flexamat; eroded bare side slopes will be track packed to fill in erosion rills, slightly regraded and covered with a compost blanket and TRM type C.
- 2. North of HWY 55 and south of HWY 813: Eroded bare side slopes will be track packed to fill in erosion rills, slightly regraded and covered with a compost blanket and or Flexamat. Existing gullies between gabion baskets will be repaired and the ditches will be lined with either riprap or TRM type C. Damaged gabion baskets will be repaired; fillcrete will be used to backfill erosion developed below G6 in the Hwy 55 north ditch and below the Hwy 813 culvert outlet.
- 3. **NE and NW corners of Tawatinaw River Bridge:** Existing drain trough and dissipation bowl located west of the bridge will be upgraded and a new drain trough and dissipation bowl will be installed on the east side of the bridge. Slope regarding and rill repairs will also be completed east of the bridge.

Client: Alberta Transportation Date: September 6, 2017 File: 13357

e-file: \\H\13357 NC004-1

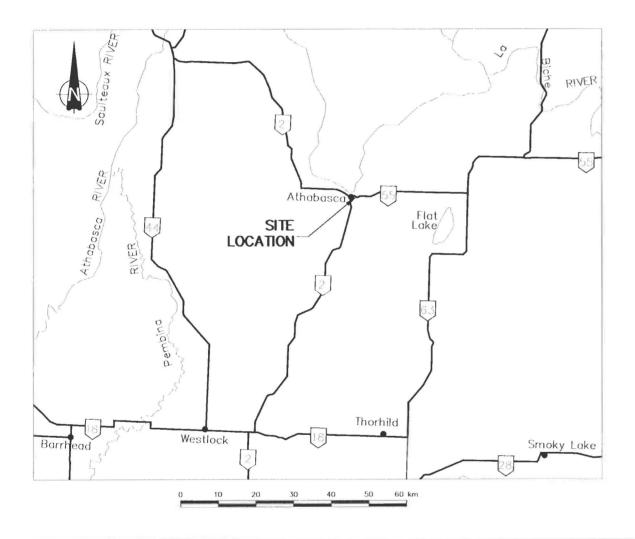


PLANS of PROJECT No. 55:10 AND 813:02 EROSION REPAIRS AND OTHER WORK (NCOO4)

HWY 55:10 km<u>0.34</u> to km<u>0.84</u>

HWY 813:02

km 0.00 to km 1.97



	INDEX	
SHEET	DESCRIPTION	DRAWING
1	SITE LOCATION MAP	RD-20772-I
2	OVERALL SITE PLAN	RD-20773-
3	DETAIL SITE PLAN-REPAIR AREA 1	RD-20774-
4	DETAIL SITE PLAN-REPAIR AREA 2	RD-20775-
5	DETAIL SITE PLAN-REPAIR AREA 3	RD-20776-
6	DETAIL SITE PLANS-REPAIR AREAS 4 AND 5	RD-20777-
7	CROSS-SECTIONS AND OTHER DETAILS	RD-20778-
8	DETAIL SITE PLAN-REPAIR AREA 6	RD-20779-
9	FLEXAMAT INSTALLATION DETAILS	RD-20780-
10	TAWATINAW RIVER BRIDGE ON HIGHWAY 55 NW DRAIN TROUGH	RD-20781-
11	TAWATINAW RIVER BRIDGE ON HIGHWAY 55 NE DRAIN TROUGH	RD-20782-

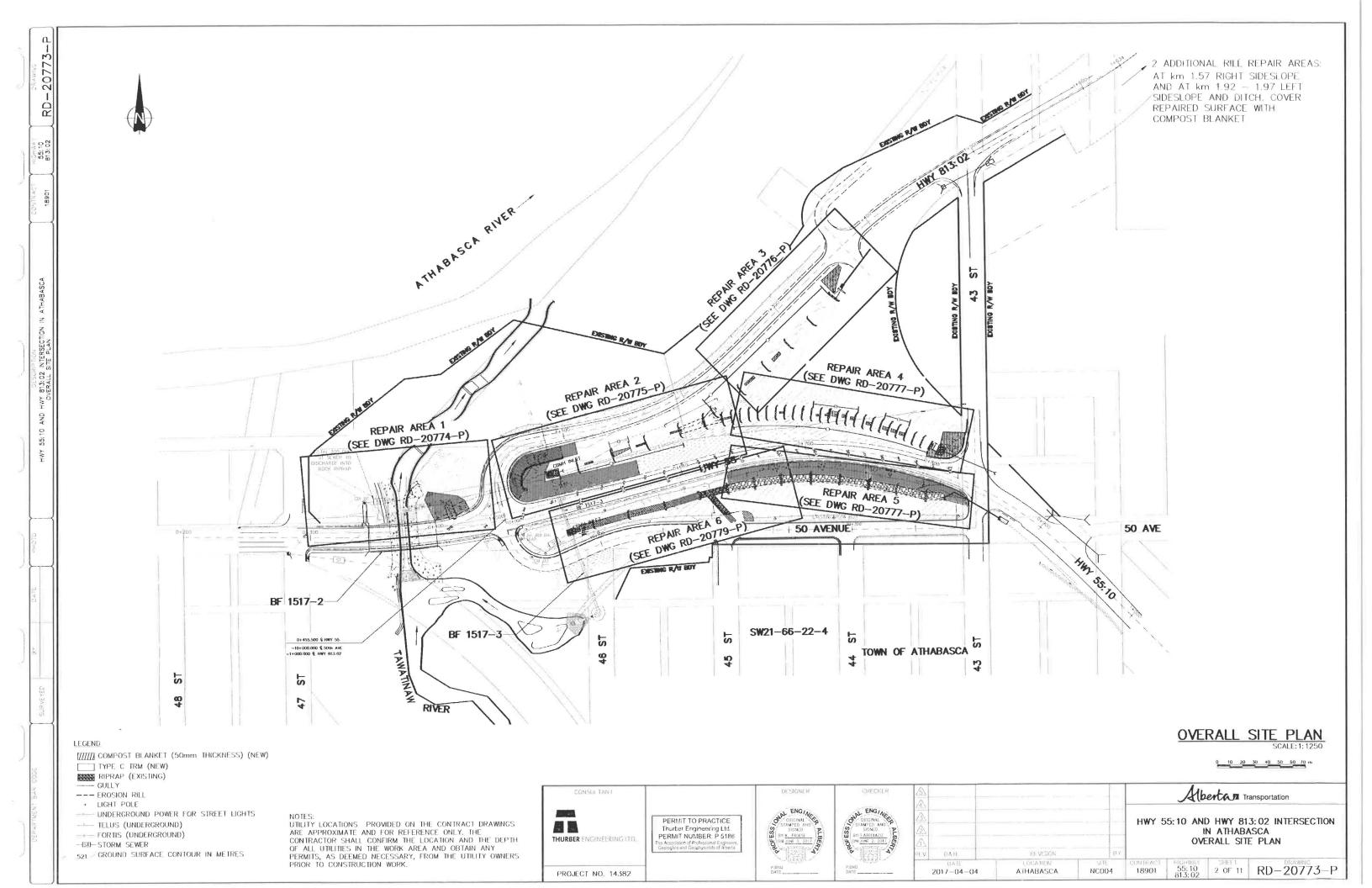
NOTES:

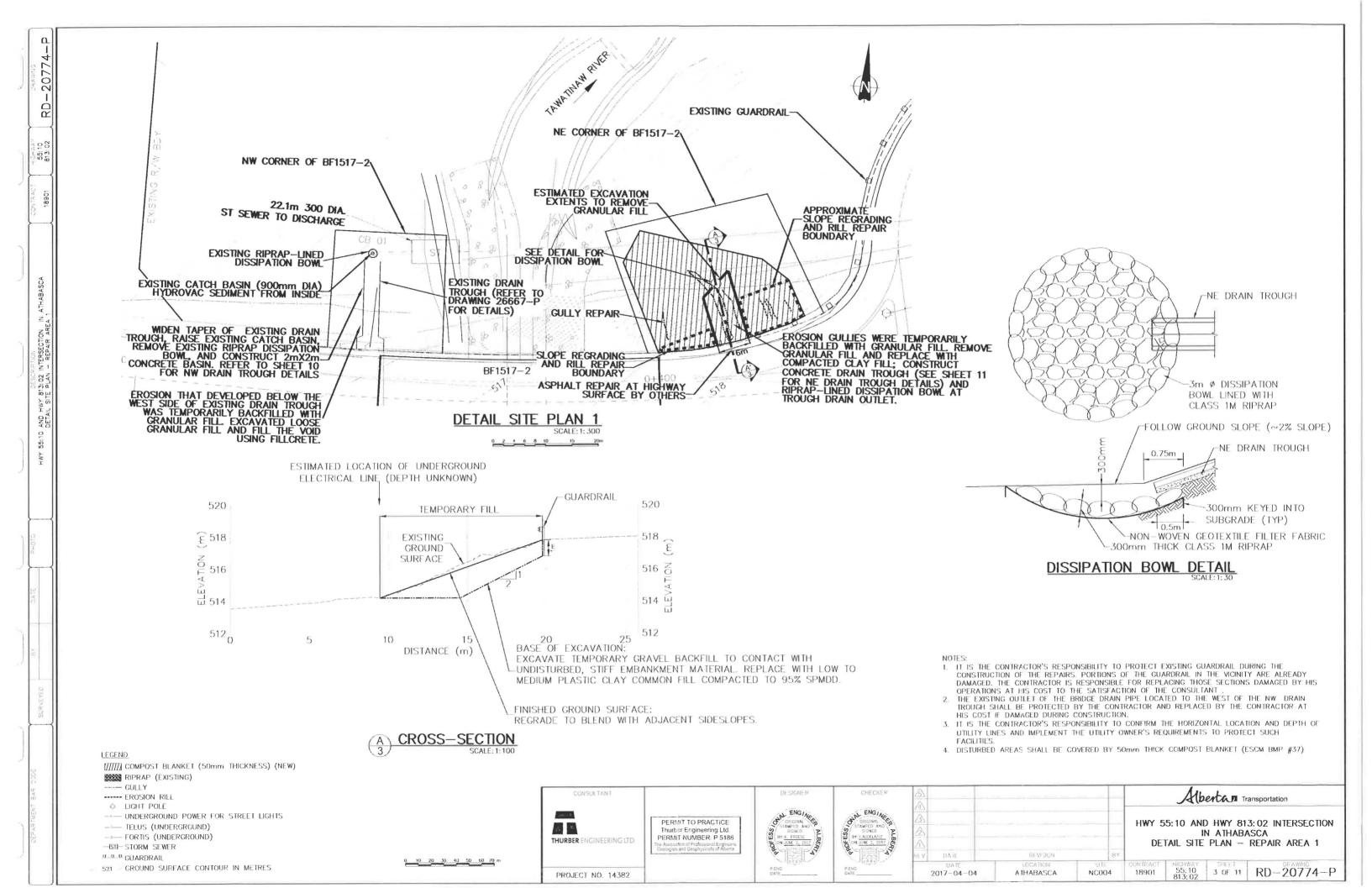
ALL LOCATIONS ARE APPROXIMATE
AND SUBJECT TO REVISION OR DELETION.
THE INFORMATION PERTAINING TO THE DATA AS SHOWN HAS BEEN
COMPILED FOR THE USE OF ALBERTA TRANSPORTATION AND UTILITIES.
NO RESPONSIBILITY WILL BE ASSUMED BY THE DEPARTMENT FOR THE
CORRECTNESS OR COMPLETENESS OF THE DATA SHOWN AND,
SHOULD ANY SUCH DATA BE FOUND INCORRECT OR INCOMPLETE,
THE CONTRACTOR SHALL HAVE NO CLAIM ON THAT ACCOUNT.
THE GRADING LIMITS ON THESE PLANS REFER
TO PERMANENT CONSTRUCTION. TEMPORARY TRANSITIONS
MAY EXTEND BEYOND THESE LIMITS.

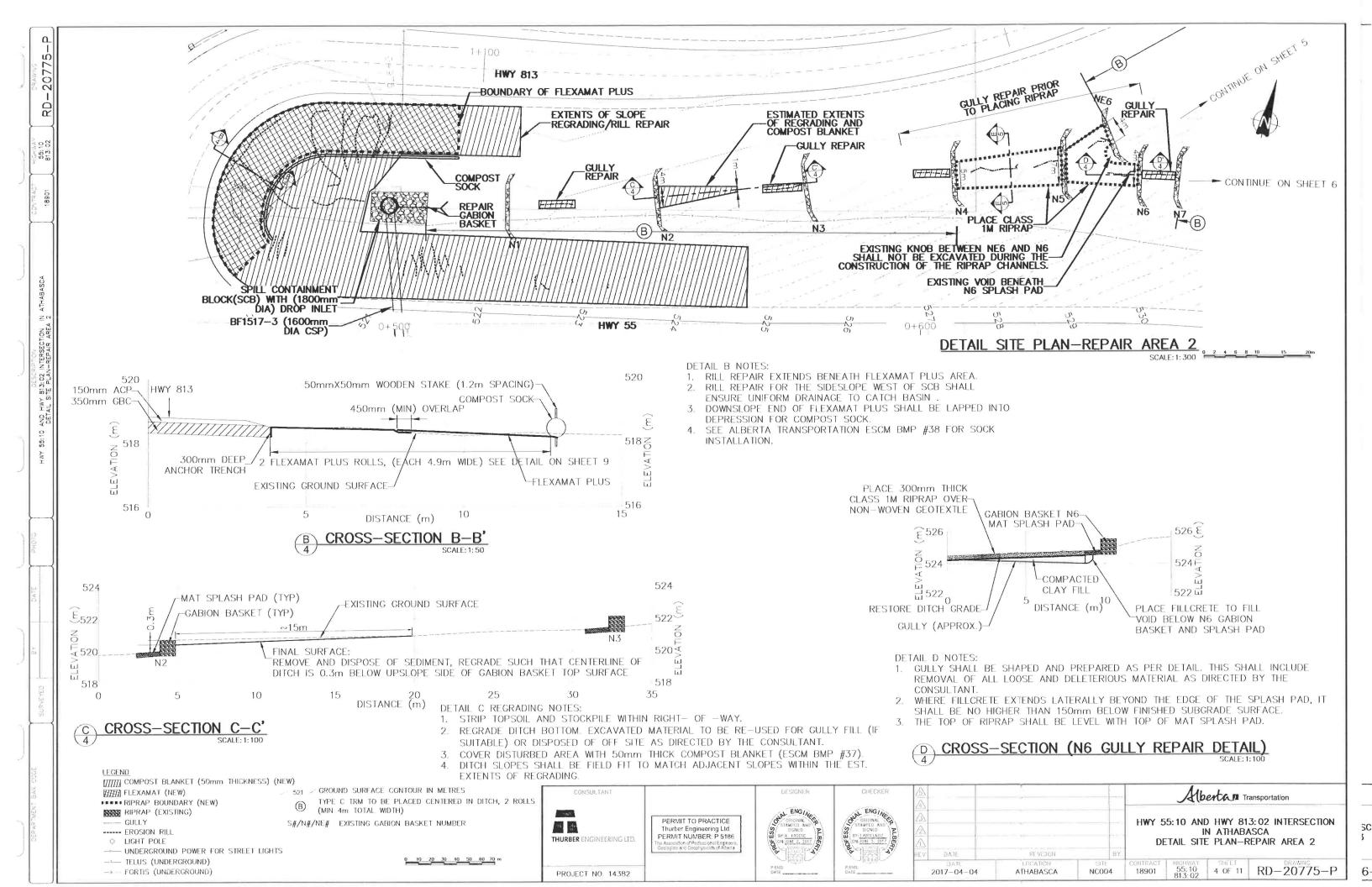
CONTRACT No. 18901 ISSUED FOR CONSTRUCTION

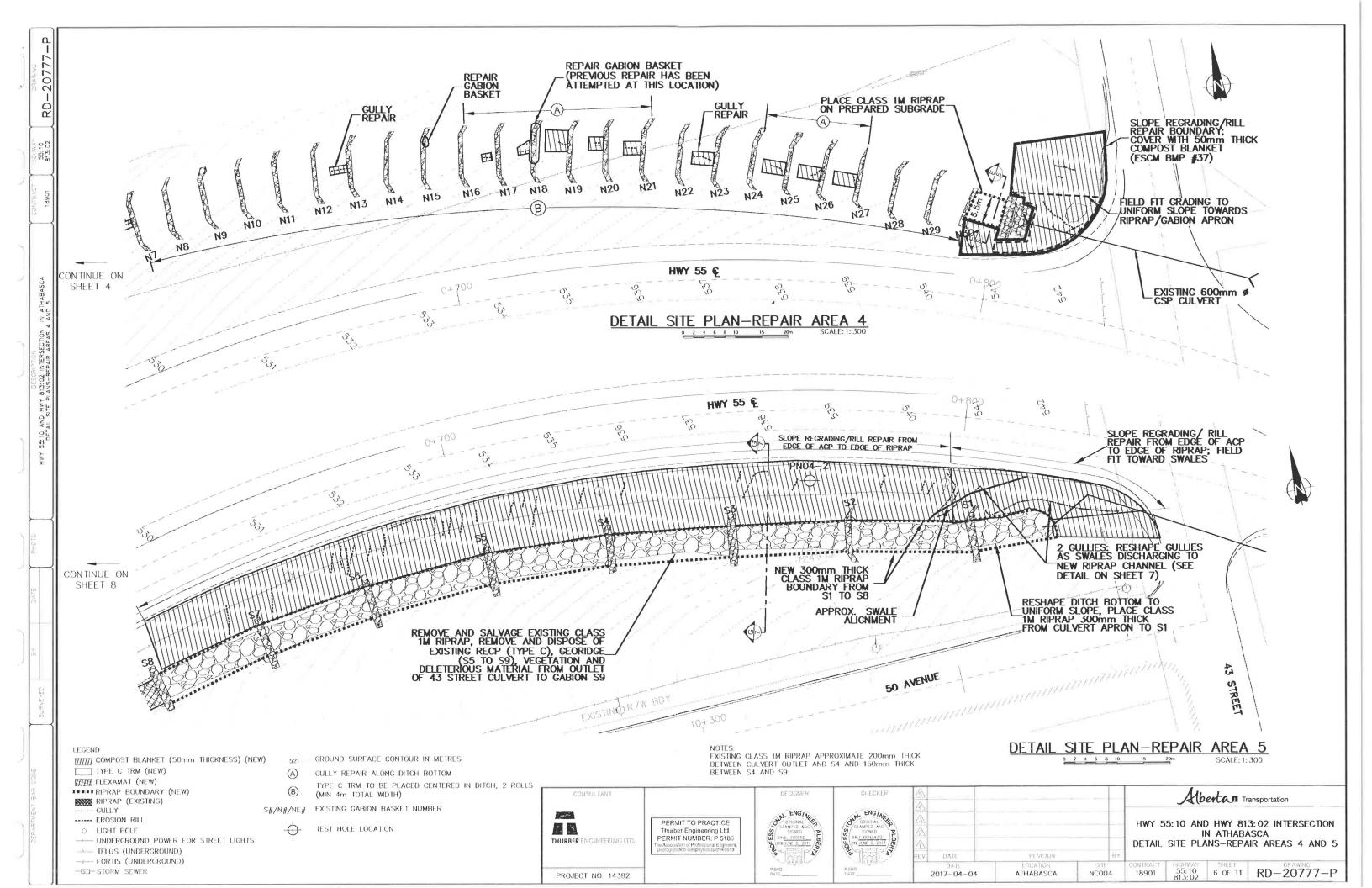


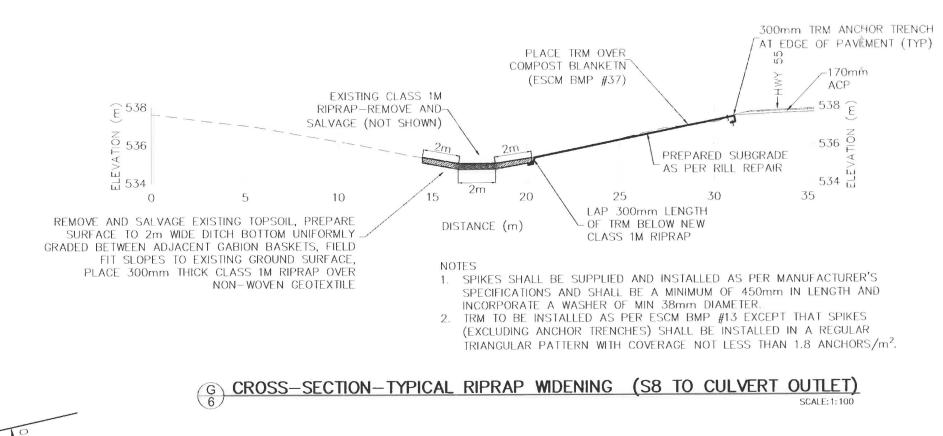


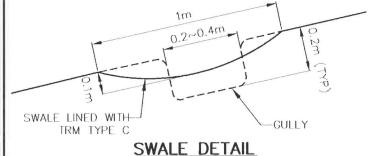












SCALE: 1:10

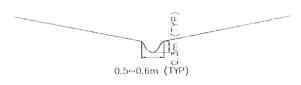
BARE GROUND UP TO 0.2m (TYP)

SCARIFY AND RECOMPACTED

RILL REPAIR

- REMOVE AND DISPOSE OF SAND ACCUMULATION WITHIN THE TREATMENT AREA SHOWN ON THE DRAWINGS, IF PRESENT.
- 2. REMOVE AND DISPOSE OF NON-WOVEN GEOTEXTILE, VEGETATION, AND DELETERIOUS MATERIALS THAT MAY BE PRESENT.
- 3. REMOVE AND SALVAGE TOPSOIL, IF PRESENT.
- 4. SCARIFY RILLED SLOPE SURFACE AND REWORK TO EVEN SURFACE. DEPTH OF SCARIFYING SHALL BE THE GREATER OF DEEPEST RILL OR 150mm.
- 5. COMPACT REWORKED SURFACE BY TRACK-PACKING WITH A DOZER WORKING IN A DIRECTION PERPENDICULAR TO FLOW DIRECTION (UP AND DOWN THE SLOPE). SEE ALBERTA TRANSPORTATION ESCM BMP#34.

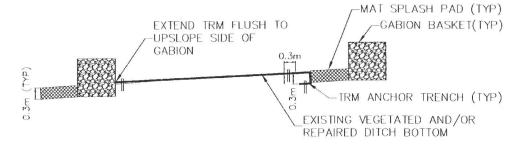
SLOPE EROSION RILL REPAIR DETAIL
SCALE:1:25



GULLY REPAIR

- 1. REMOVE AND SALVAGE TOPSOIL, IF PRESENT.
- EXCAVATE LOOSE AND OVERHANGING MATERIAL FROM ERODED SURFACE AND SHAPE.
- 3. PLACE COMPACTED IMPORTED CLAY IN 150mm LIFTS TO MATCH ADJACENT GRADE AND AS DIRECTED BY THE CONSULTANT.
- COVER FILL WITH SURFACE TREATMENT SPECIFIED ON DETAIL SITE PLAN DRAWINGS.

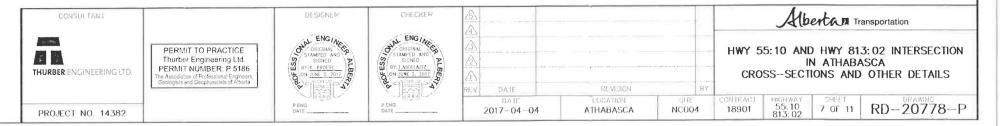
GULLY REPAIR DETAIL
SCALE: 1:50

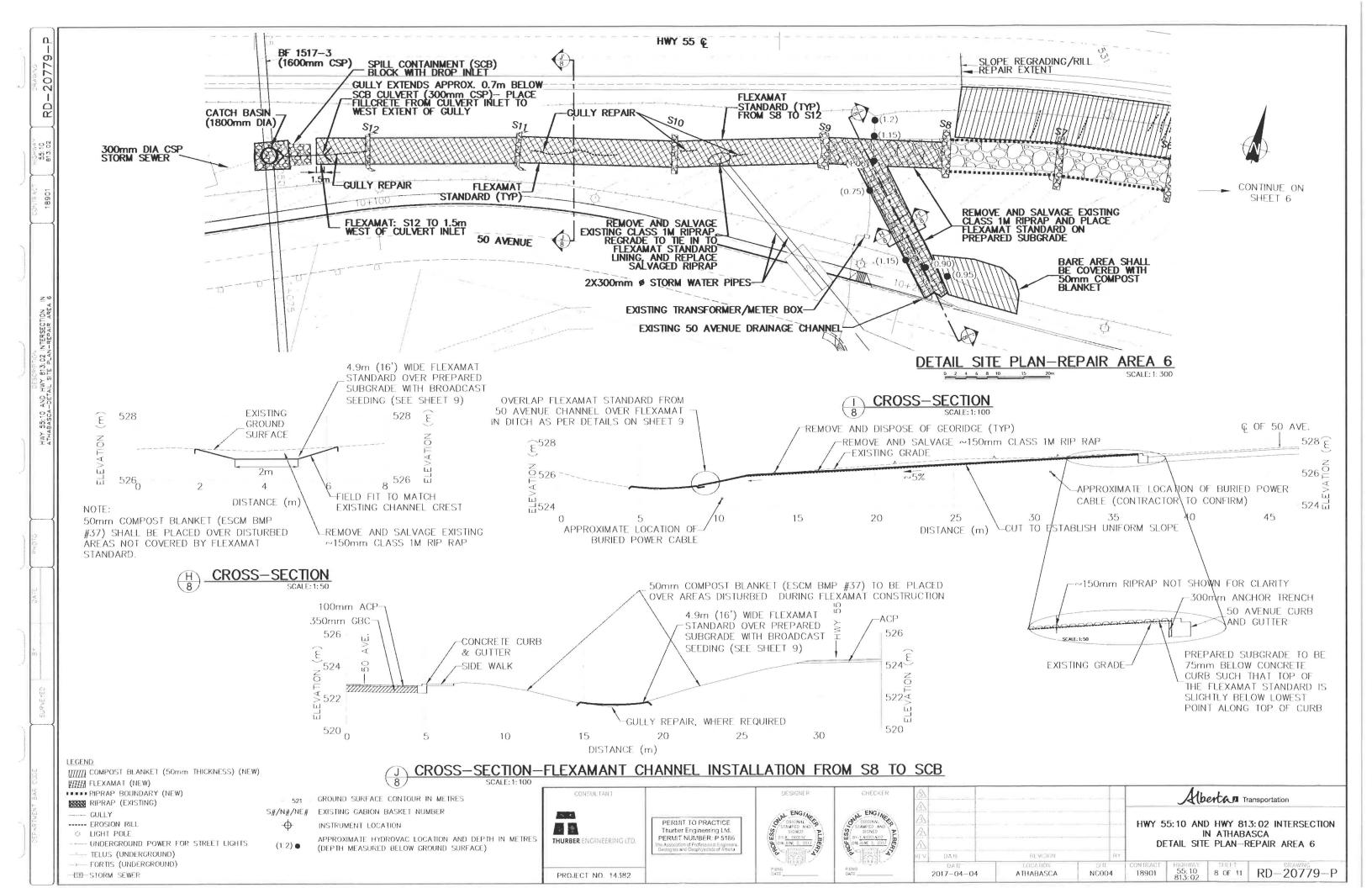


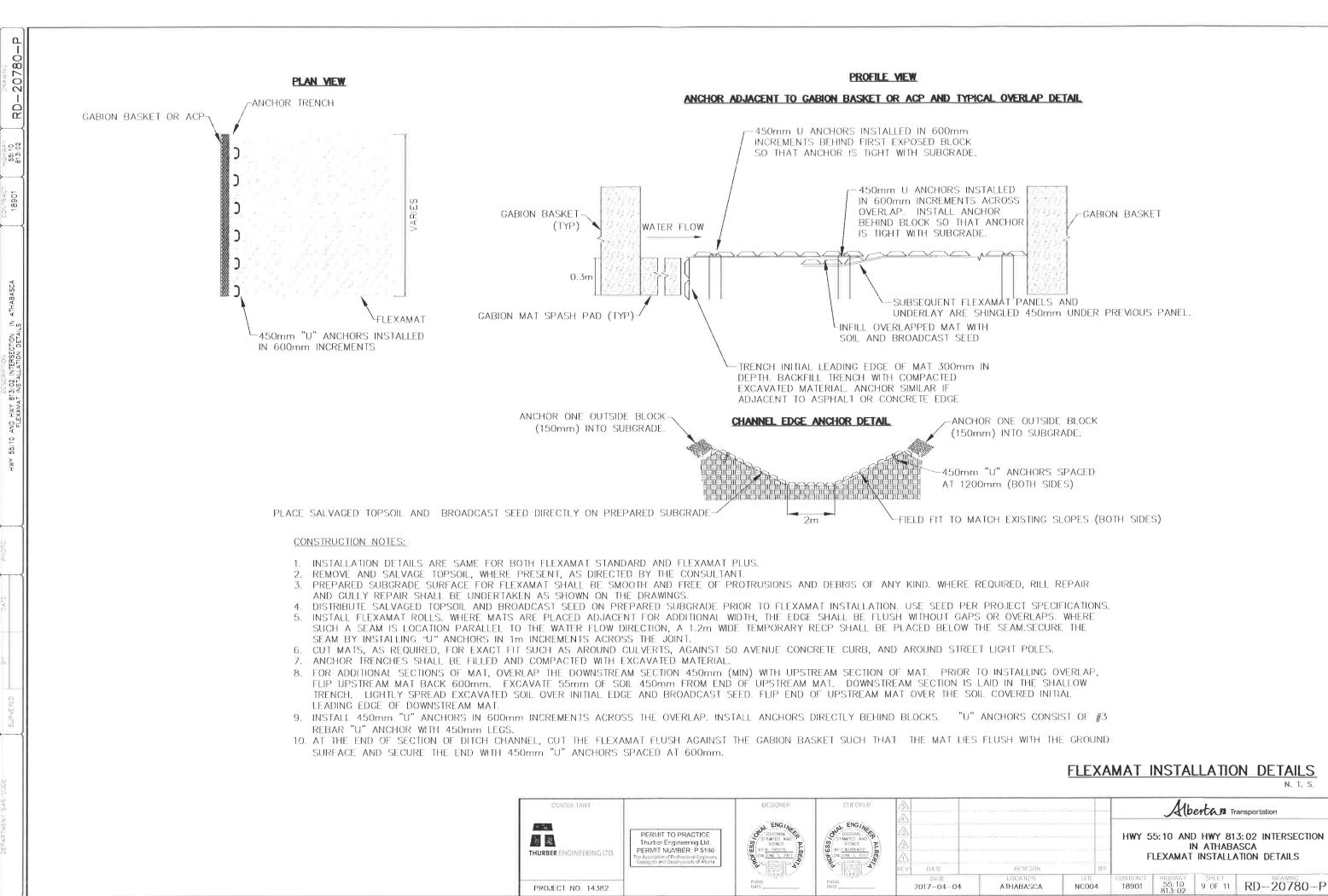
NOTES

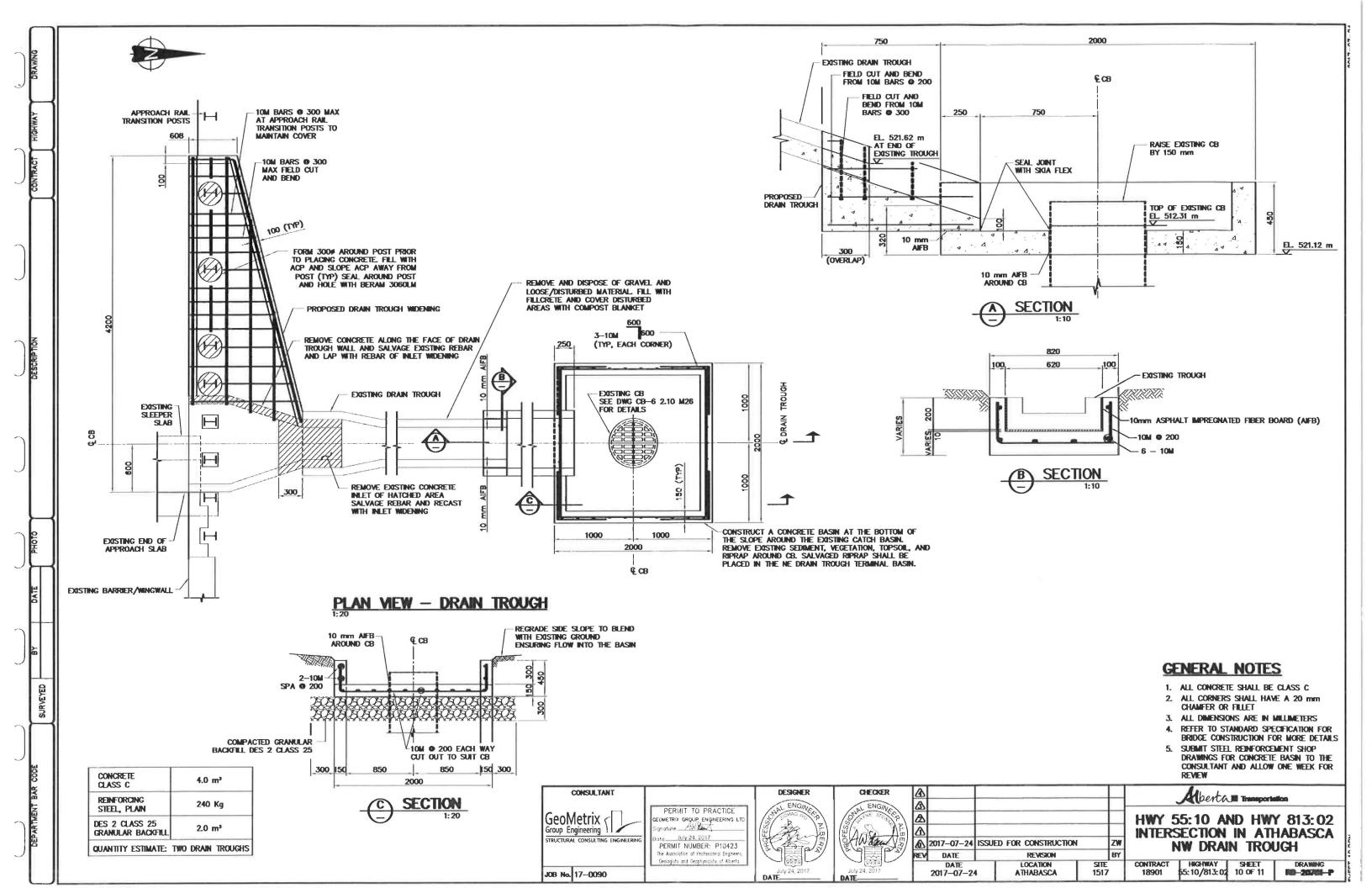
- SPIKES SHALL BE SUPPLIED AND INSTALLED AS PER MANUFACTURER'S SPECIFICATIONS AND SHALL BE A MINIMUM OF 450mm IN LENGTH AND INCORPORATE A WASHER OF MIN 38mm DIAMETER.
- 2. TRM TO BE INSTALLED AS PER ESCM BMP #13 EXCEPT THAT SPIKES (EXCLUDING THOSE IN THE ANCHOR TRENCHES) SHALL BE INSTALLED IN A REGULAR TRIANGULAR PATTERN WITH COVERAGE NOT LESS THAN 1.2 ANCHORS/m².
- 3. EXISTING VEGETATION (WHERE PRESENT) SHALL NOT BE DISTURBED AND THE TRM INSTALLED DIRECTLY OVER TOP OR OVER COMPOST BLANKET AT SPECIFIED LOCATION ON THE DRAWINGS.
- ANCHOR TRENCH TO BE BACKFILLED WITH COMPACTED EXCAVATION SOIL BROADCAST SEED OVER TOP OF BACKFILL.

TYPE C TRM DITCH COVER DETAIL SCALE:1:50









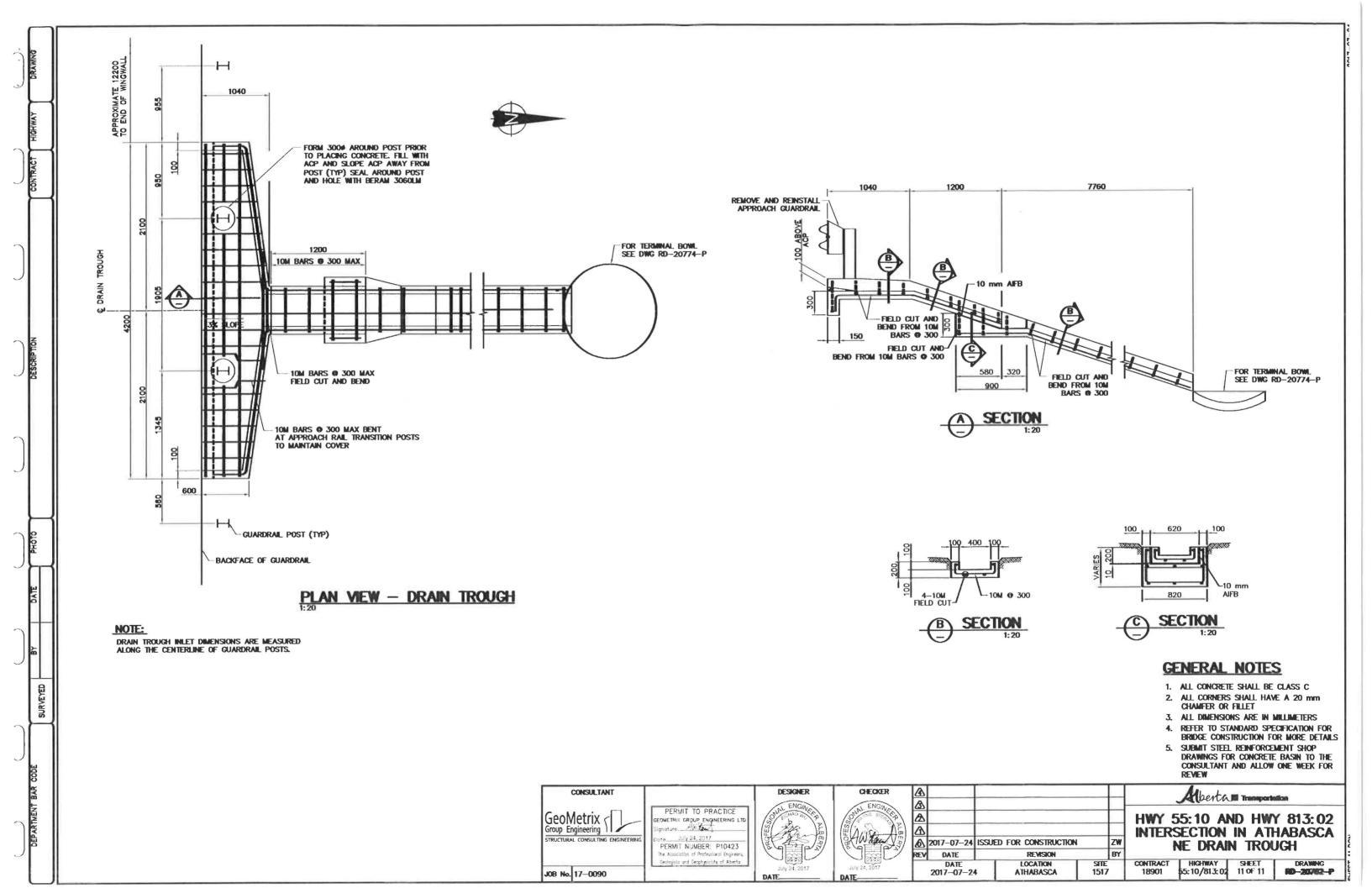








Photo #1.
Hwy 55 South Ditch.
Looking west at
existing riprap
channel from the
east of G1.



Photo #2. Hwy 55 South Ditch: Looking west at the east end of the existing riprap channel. Note the presence of bare side slopes and an erosion gully on the slope surface







Photo #3.
Hwy 55 South Ditch:
Looking east at the
existing riprap
channel



Photo #4. Hwy 55 South Ditch: Looking east at riprap between G9 and G8







Photo #5.

Looking northwest at the existing Class 1 M Riprap lined swale extending from the end of the concrete curb and gutter along the north edge of 50 Avenue to the south ditch of Hwy 55; swale is not well defined and a few areas are bare of riprap.



Photo #6.

Looking southeast at the outlets of the storm water pipes; signs of erosion near the outlets of the pipes







Photo #7. Hwy 55 south ditch: looking east at signs of erosion between G10 and G11



Photo #8.
Hwy 55 south ditch:
looking east at
erosion gully
between G 12 and
the spill containment
ditch block







Photo #9. Looking southeast at a bare sandy slope on the south side slope of Hwy 55



Photo #10. Highway 55 south ditch: Ponding water within a low area to the west of the 1200 mm diameter manhole.







Photo #11.
Looking west at the northeast corner of the intersection of Hwy 55 and 813.
Bare vegetation and erosion rills in this area



Photo #12.
Looking north at
deep erosion gullies
developed within NE
corner of the bridge;
erosion gully was
backfilled with gravel
in 2016







Photo #13.
Drain trough on the northwest side of bridge crossing Tawatinaw River.
Erosion developed below and on the west facing side the of the drain.
Sediment accumulated around catch basin/dissipation bowl.



Photo #14. Highway 55 north ditch: Looking east at bare side slopes







Photo #15.
Hwy 55 North Ditch:
Looking north from
the manhole location
at erosion rills and
bare slopes



Photo #16. Hwy 55 North Ditch: Erosion gully between G2 and G3







Photo #17. Hwy 55 North Ditch: Erosion gully between G4 and G5



Photo #18.
Hwy 55 North
Ditch: Erosion gully
between G5 and
G6; note the gap
developed below
G6







Photo #19.
Hwy 55 North
Ditch: looking west
from the west side
slope of 43 Street.
Bare ground
surface and erosion
rills in the side
slope